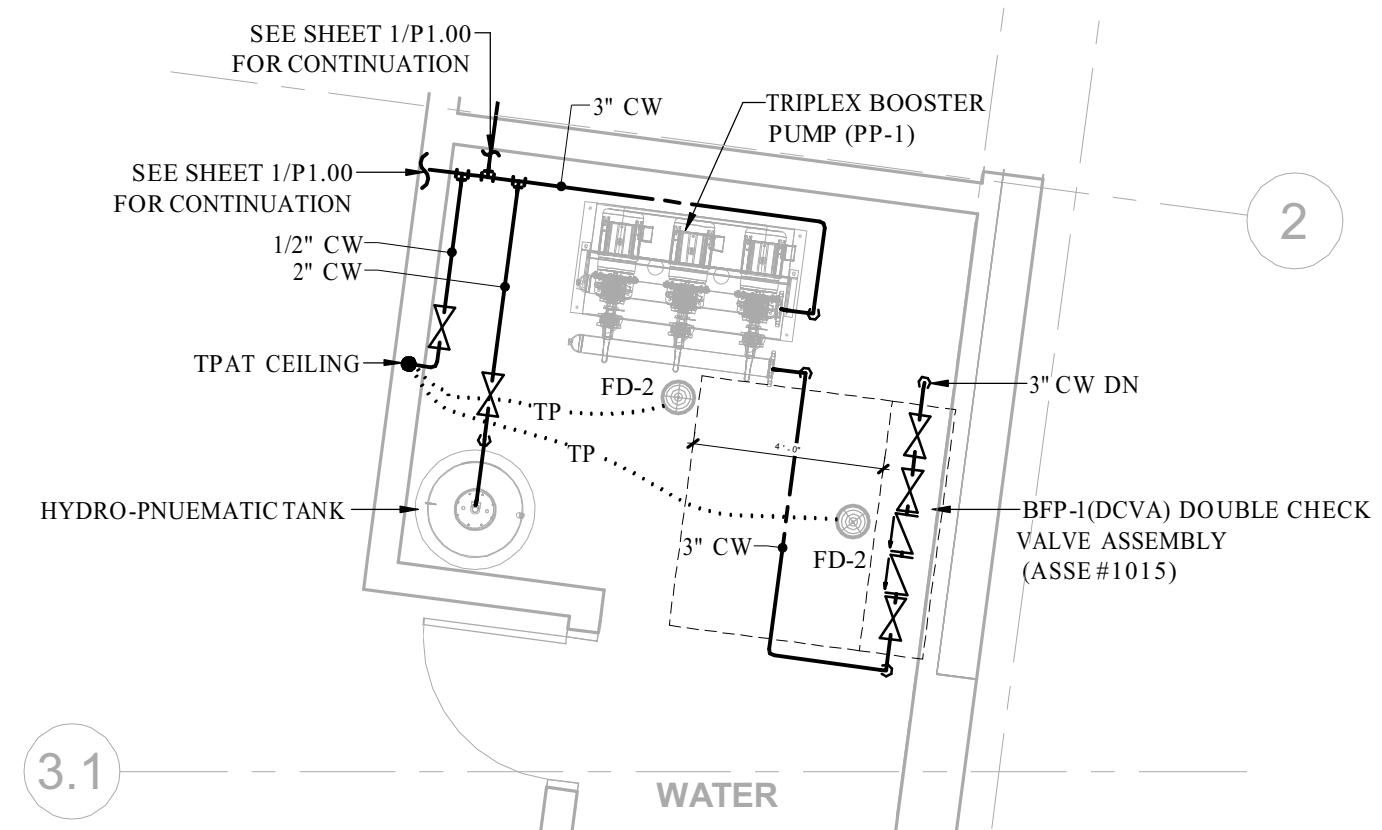




1 PARKING LEVEL PLAN - PLUMBING
1/8" = 1'-0"

2 PLUMBING ENLARGED PLAN - RR ROOM
1/4" = 1'-0"



3 PLUMBING ENLARGED PLAN - WATER ROOM
1/4" = 1'-0"

GENERAL SHEET NOTES

A.

KEYED SHEET NOTES

L.

GOVERNMENT OF THE DISTRICT OF COLUMBIA
PERMIT OPERATIONS DIVISION
PLANS APPROVED
 Permit No. FD1900028 Date 05/21/19

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CQA#21811

Drawing Title

P1 LEVEL PLAN - PLUMBING

Scale As indicated
 Date 02/08/19
 Drawn By CQA
 Checked By CQA

P1.00
 District of Columbia
 CASE NO.20183
 EXHIBIT NO.54

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CQA#21811
 Drawing Title

PLUMBING DOMESTIC WATER RISER DIAGRAM

Scale 1/8" = 1'-0"
 Date 02/08/19
 Drawn By CQA
 Checked By CQA

P5.01

NOT FOR CONSTRUCTION



3" DOMESTIC WATER SERVICE - (NVL: 223.88) (21 SFU=86GPM) SEE CIVIL DRAWINGS FOR CONTINUATION

1 DOMESTIC WATER RISER DIAGRAM
 P5.01 NO SCALE

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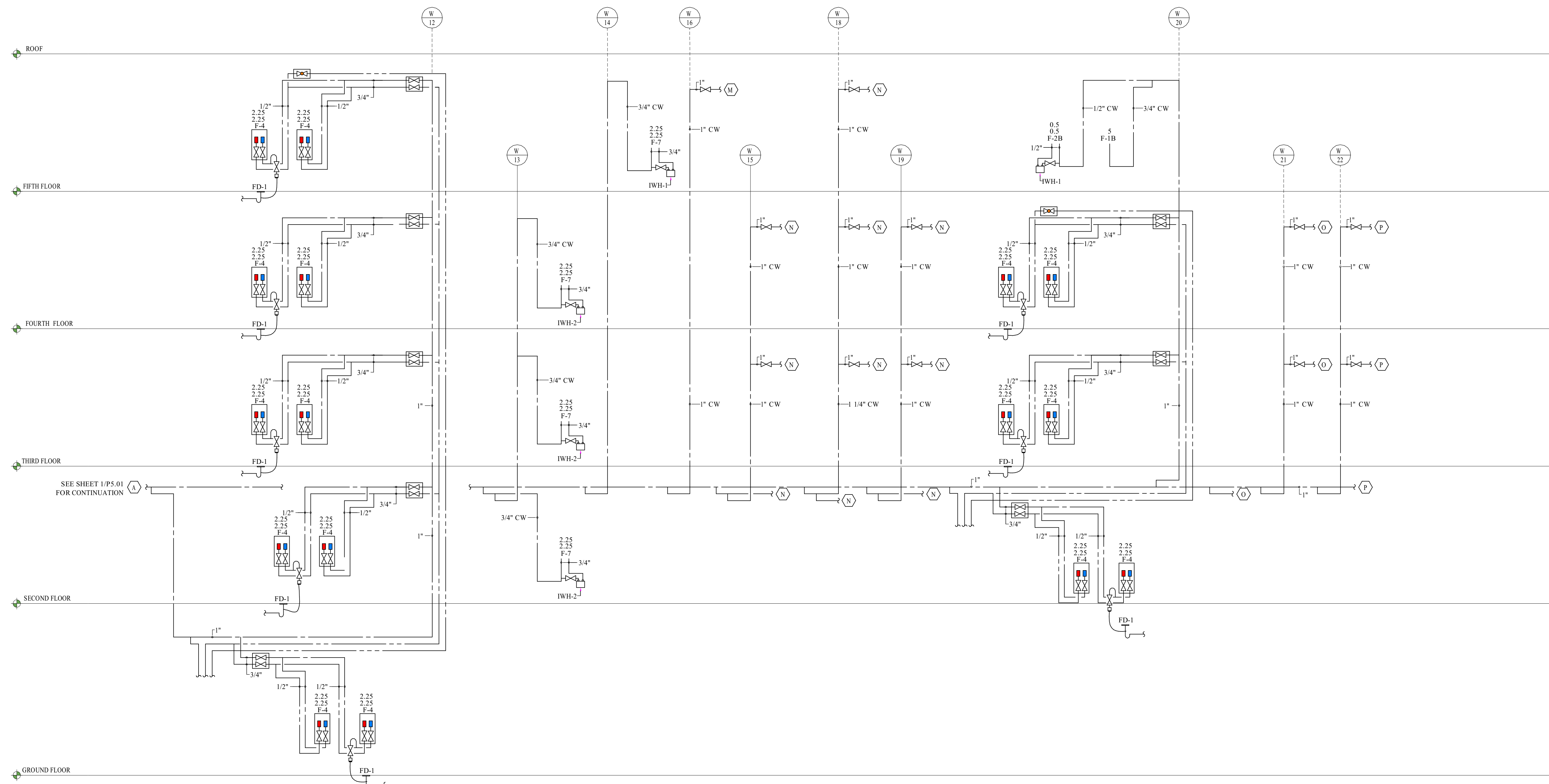
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PLUMBING DOMESTIC WATER RISER DIAGRAM

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 Date: 02/08/19
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P5.02

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SEE SHEET 1/PS 01 FOR CONTINUATION

1 DOMESTIC WATER RISER DIAGRAM.
 P5.02 NO SCALE

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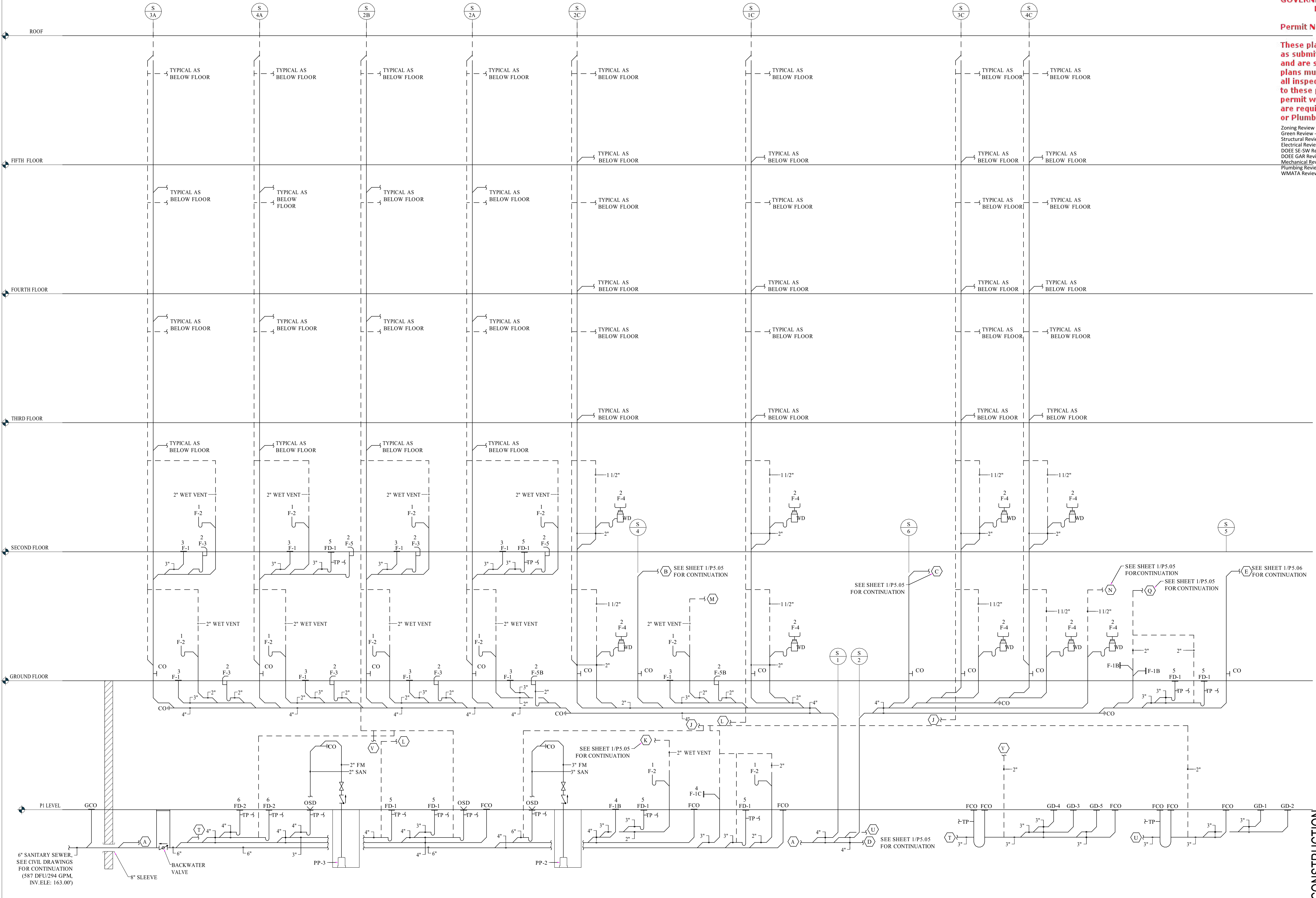
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PLUMBING SANITARY RISER DIAGRAM

Scale 1/8" = 1'-0"
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P5.04

NOT FOR CONSTRUCTION



1 PLUMBING SANITARY RISER DIAGRAM
 P5.04 NO SCALE

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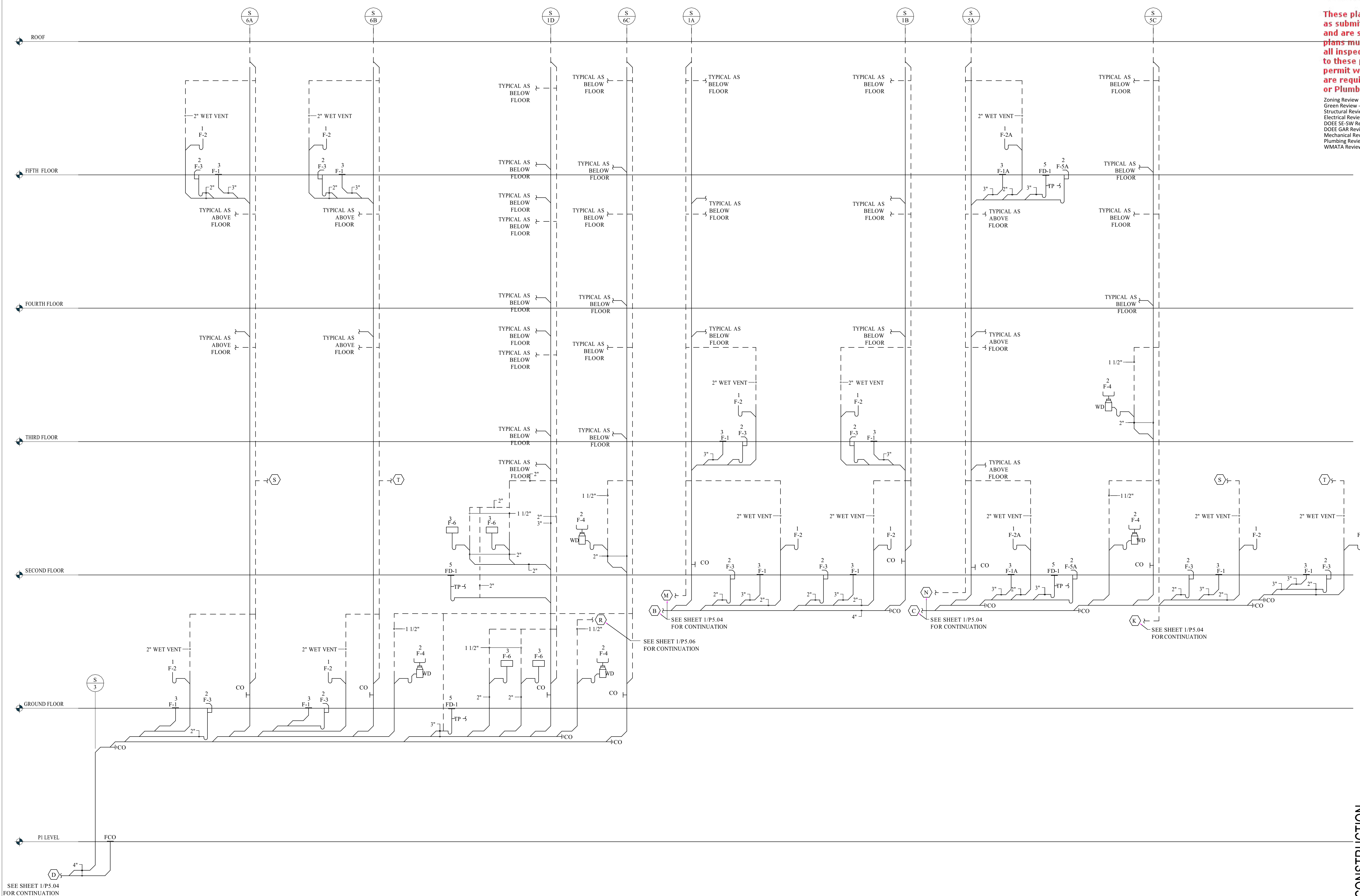
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Scale: 1/8" = 1'-0"
 Date: 02/08/19
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P5.05

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1 PLUMBING SANITARY RISER DIAGRAM.
 P5.05 NO SCALE

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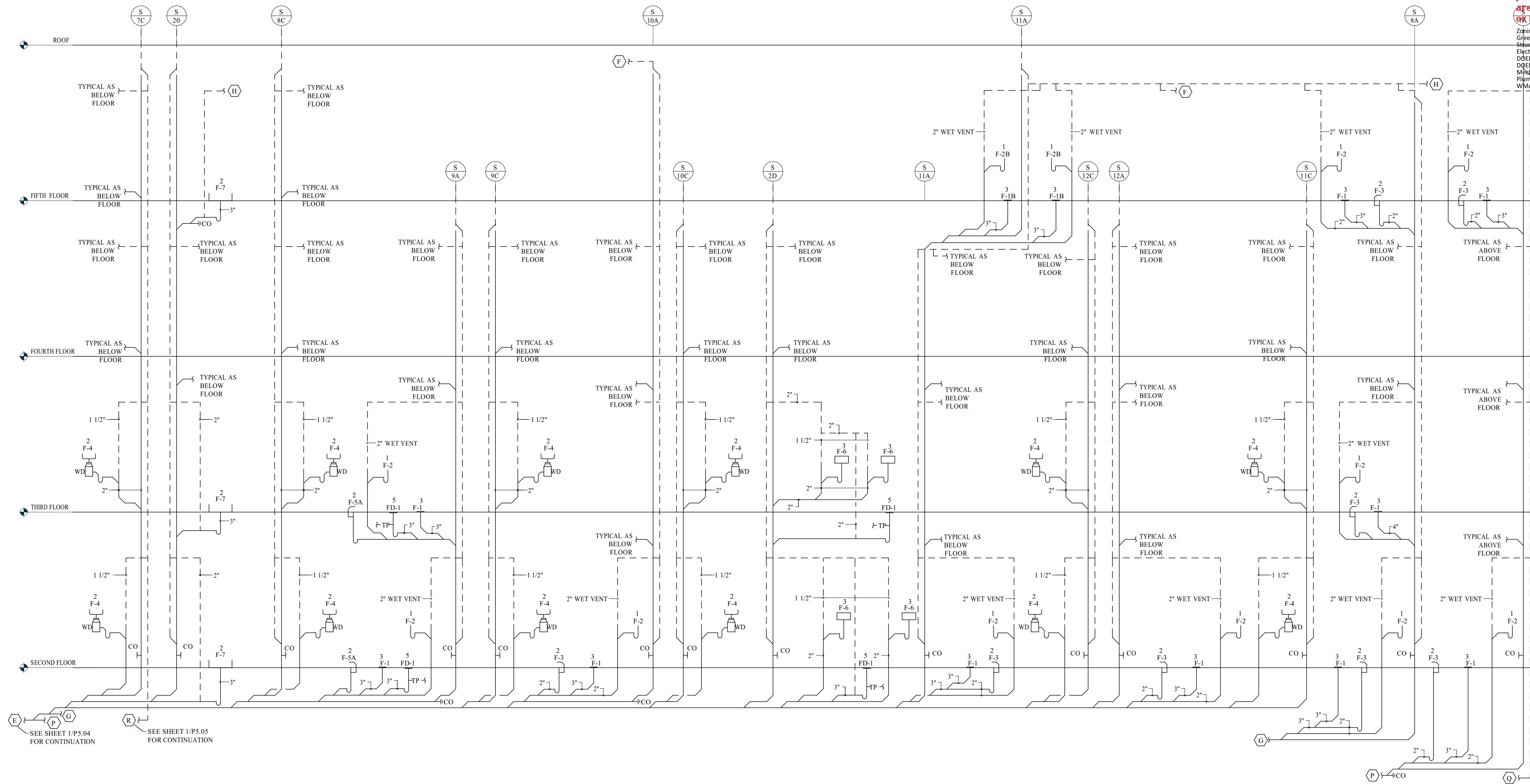
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PLUMBING SANITARY RISER DIAGRAM

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P5.06



SEE SHEET 1/P5.04 FOR CONTINUATION
 SEE SHEET 1/P5.05 FOR CONTINUATION

1 PLUMBING SANITARY RISER DIAGRAMS
 P5.06 NO SCALE

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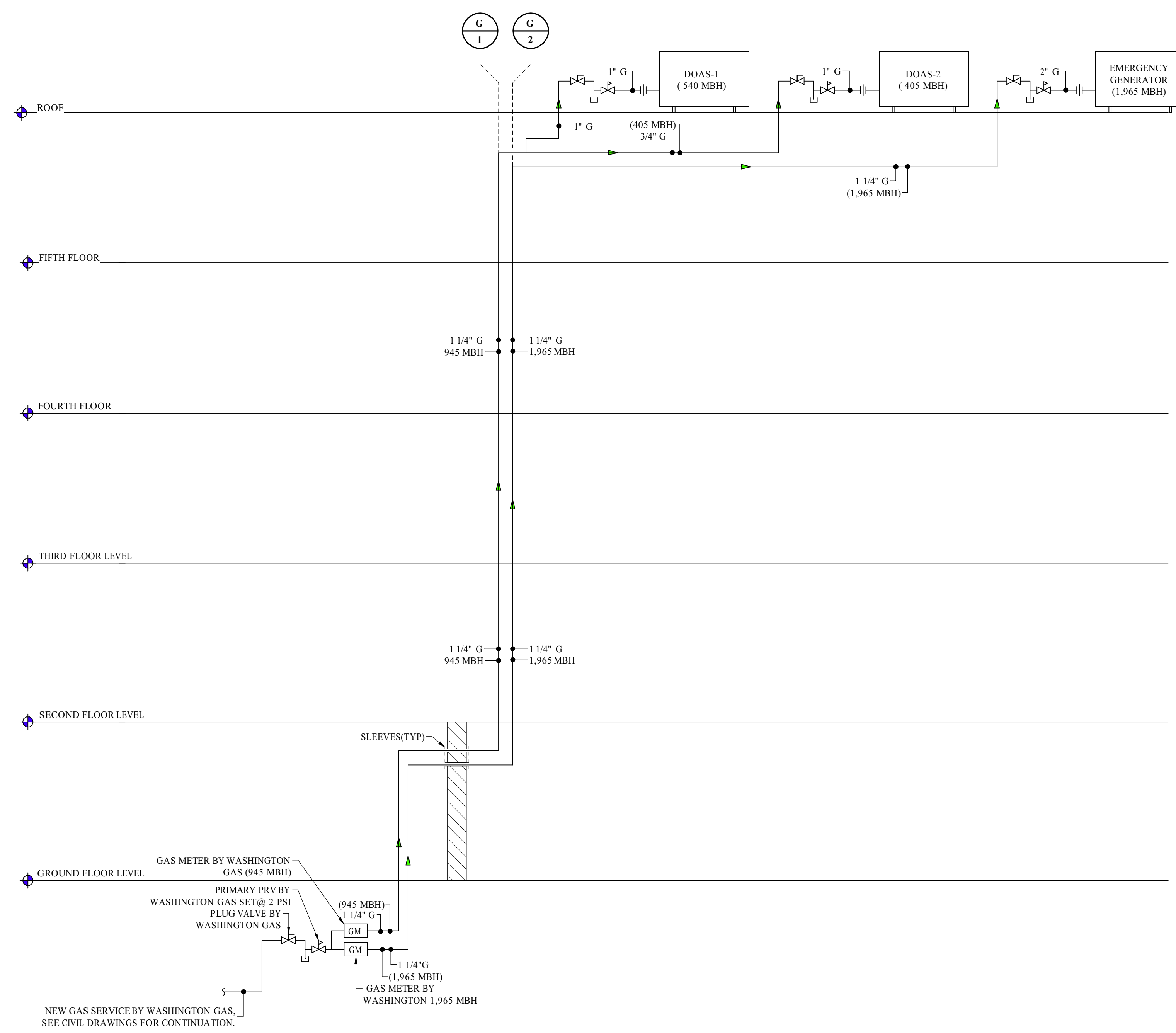
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Drawing Title

PLUMBING GAS RISER DIAGRAM

Scale	NTS	Drawn By	CQA
Date	02/08/19	Checked By	CQA

P5.07



- | | |
|---|---|
| <p>GAS SERVICE TO (DOAS-1,DOAS-2)</p> <p>MEDIUM PRESSURE SYSTEM #1: FROM GAS METER TO SECONDARY PRV:
 1. SIZING BASED ON SCHEDULE 40 BLACK STEEL.
 2. SYSTEM PRESSURE: 2 PSI.
 3. MAXIMUM LENGTH OF RUN: NOT TO EXCEED 350 FEET OF LENGTH.
 4. MAXIMUM PRESSURE DROP: NOT TO EXCEED 1 PSI.</p> <p>LOW PRESSURE SYSTEM #1: FROM SECONDARY PRV TO APPLIANCES:
 1. SIZING BASED ON SCHEDULE 40 BLACK STEEL.
 2. SYSTEM PRESSURE: 10.5 INCHES WC.
 3. MAXIMUM LENGTH OF RUN: NOT TO EXCEED 20 FEET OF LENGTH.
 4. MAXIMUM PRESSURE DROP: NOT TO EXCEED 0.5 INCHES WC.
 5. LOCATE SECONDARY PRV AT APPLIANCE.</p> | <p>GAS SERVICE TO (EMERGENCY GENERATOR)</p> <p>MEDIUM PRESSURE SYSTEM #1: FROM GAS METER TO SECONDARY PRV:
 1. SIZING BASED ON SCHEDULE 40 BLACK STEEL.
 2. SYSTEM PRESSURE: 2 PSI.
 3. MAXIMUM LENGTH OF RUN: NOT TO EXCEED 250 FEET OF LENGTH.
 4. MAXIMUM PRESSURE DROP: NOT TO EXCEED 1 PSI.</p> <p>LOW PRESSURE SYSTEM #1: FROM SECONDARY PRV TO APPLIANCES:
 1. SIZING BASED ON SCHEDULE 40 BLACK STEEL.
 2. SYSTEM PRESSURE: 10.5 INCHES WC.
 3. MAXIMUM LENGTH OF RUN: NOT TO EXCEED 20 FEET OF LENGTH.
 4. MAXIMUM PRESSURE DROP: NOT TO EXCEED 0.5 INCHES WC.
 5. LOCATE SECONDARY PRV AT APPLIANCE.</p> |
|---|---|

NATURAL GAS EQUIPMENT DEMAND								
ID	FIXTURE	NEW EQUIPMENT MBH/H RATING	QTY	TOTAL LOAD (MBH)	REQUIRED INLET PRESSURE	PIPE SIZE CONNECTION	GAS SERVICE	EQUIPMENT LOCATION
DOAS - 1	DEDICATED OUTDOOR AIR SYETEM	540	1	540	6" - 10.5" W.C.	1"	2 PSI	ROOF
DOAS - 2	DEDICATED OUTDOOR AIR SYETEM	405	1	405	6" - 10.5" W.C.	1"	2 PSI	ROOF
-	EMERGENCY GENERATOR	1,965	1	1,965	7" - 11" W.C	2"	2 PSI	ROOF
TOTAL				2,910	-	-	-	-

1 GAS RISER DIAGRAM
 P5.07 NO SCALE

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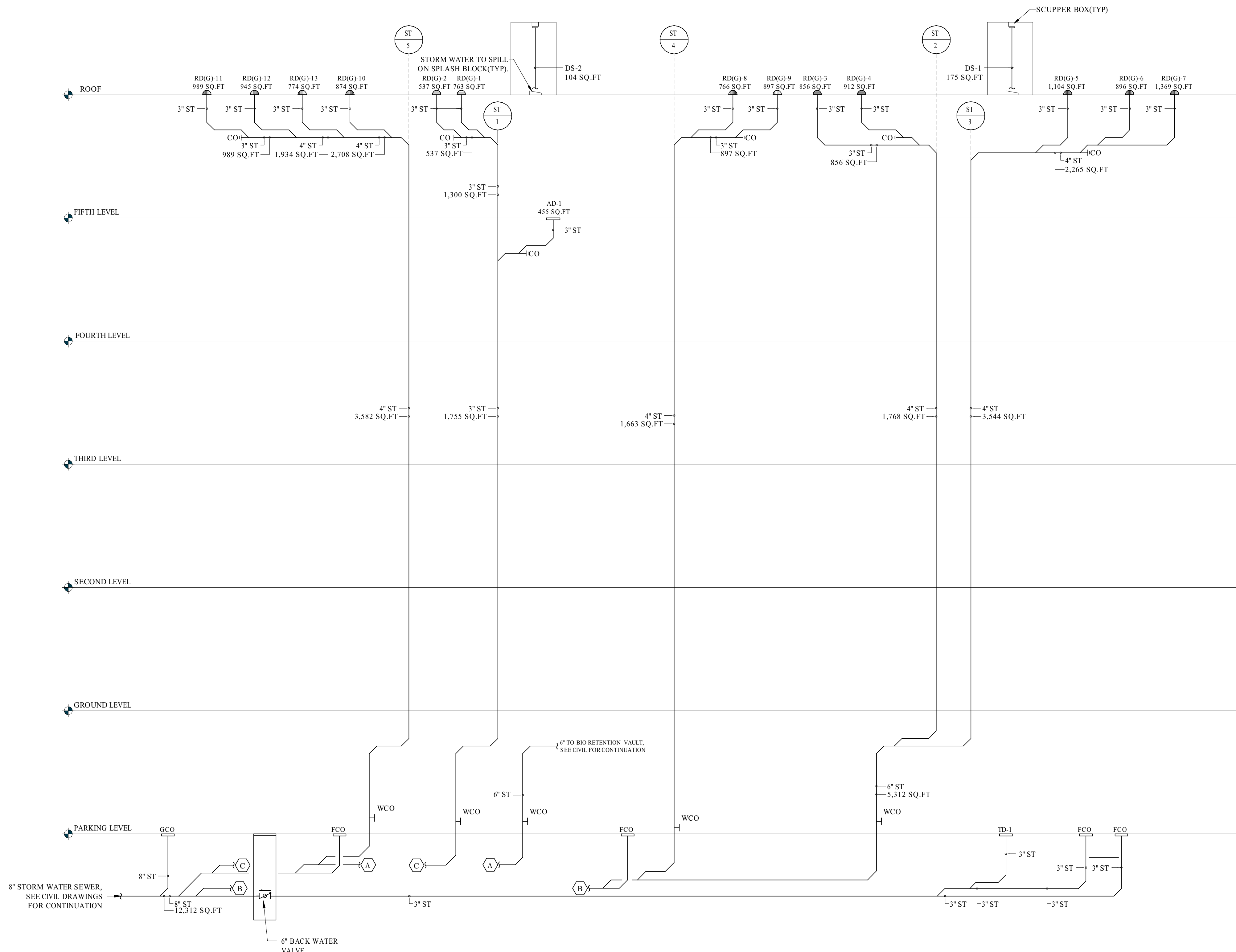
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PLUMBING STORM RISER DIAGRAM

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P5.08

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1 STORM RISER DIAGRAM
 P5.08 NO SCALE

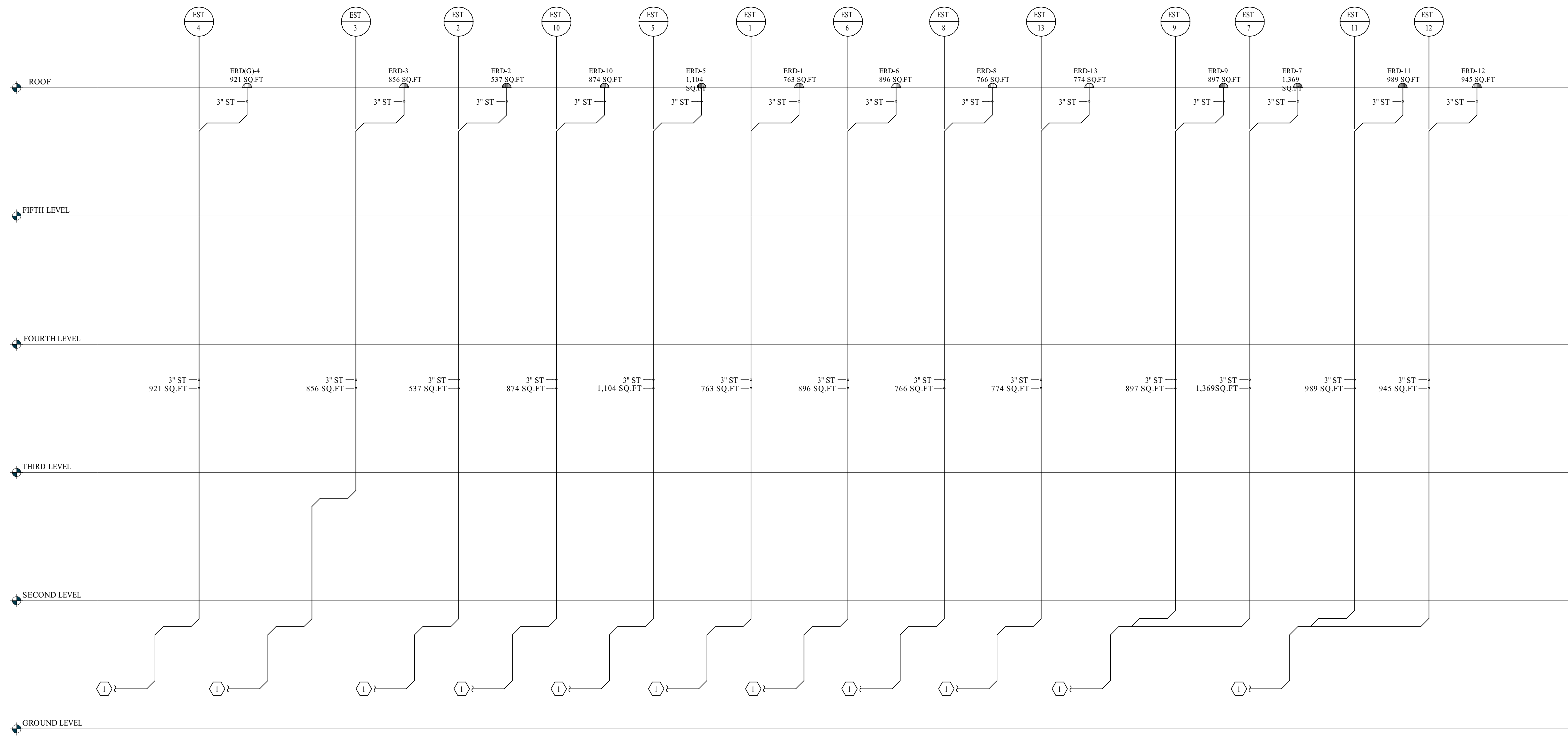
KEYED SHEET NOTES

1. EMERGENCY STORM DRAINAGE TO GRADE, PIPE 18"-24" ABOVE FINISHED GRADE. SPILL TO SPLASH BLOCK, COORDINATE WITH CIVIL PLANS FOR GRADE ELEVATION.

GOVERNMENT OF THE DISTRICT OF COLUMBIA
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 PLANS APPROVED
 Permit No. PD1900028 Date 05/21/19

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1
 P5.09 EMERGENCY STORM RISER DIAGRAM
 NO SCALE

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GENERAL

THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE FEDERAL AND STATE CODES, STANDARDS, REGULATIONS AND LAWS.

IN THE CASE OF CONFLICTS BETWEEN THE NOTES, DETAILS AND SPECIFICATIONS, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN.

THE CONTRACTOR SHALL NOT MAKE ANY DEVIATIONS FROM THE DESIGN DOCUMENTS WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD.

THE CONTRACTOR SHALL VERIFY IN FIELD ALL EXISTING CONDITIONS AND REPORT ANY CONFLICTS WITH THE DESIGN DOCUMENTS FOR CLARIFICATIONS PRIOR TO WORK.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH CONTRACT DOCUMENTS RELATED TO OTHER TRADES. THE CONTRACTOR IS RESPONSIBLE TO CHECK AND COORDINATE AND REPORT CONFLICTS IF THERE ARE ANY PRIOR TO WORK.

TYPICAL DETAILS MAY NOT NECESSARILY BE INDICATED ON THE PLANS BUT STILL APPLY. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT IS REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT SIMILAR PLACES SHALL BE REPEATED AND INCLUDED IN THE PROJECT. THE CONTRACTOR HAS THE OPTION TO SUBMIT ALTERNATE DETAILS WITH SIGNED AND SEALED CALCULATIONS TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO PROCEEDING WITH WORK.

THE STRUCTURE WAS ANALYZED AND DESIGNED BY THE ENGINEER OF RECORD CONSIDERING ITS COMPLETED STATE ONLY. THE CONTRACTOR SHALL MAINTAIN THE STABILITY OF THE BUILDING DURING CONSTRUCTION AND ANY TEMPORARY ERECTION BRACING SHALL REMAIN IN PLACE UNTIL ALL PERMANENT SYSTEM HAS BEEN COMPLETELY INSTALLED AND REACH ITS DESIGN STRENGTH.

THE CONTRACTOR SHALL CONSIDER ALL ASPECTS OF CONSTRUCTION SEQUENCING. CONSIDERATIONS SHALL INCLUDE BUT NOT BE LIMITED TO STEEL ERECTION AND CONCRETE PLACEMENT, CRANE REQUIREMENTS, TEMPORARY SHORING, BRACING/STRENGTHENING, TEMPORARY CONSTRUCTION LOADS, SAFETY PROCEDURES, TEMPERATURE CHANGE, AND MOISTURE EFFECTS.

REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL, ELEVATOR, OR OTHER SPECIALTY ENGINEERING DRAWINGS FOR DIMENSIONS NOT SHOWN, INCLUDING BUT NOT LIMITED TO: SIZE AND LOCATION OF CURBS, EQUIPMENT HOUSEKEEPING PADS, WALL AND FLOOR OPENINGS, BLOCKOUTS, FLOOR DEPRESSIONS, SUMPS, DRAINS, ANCHOR BOLTS, EMBEDDED ITEMS, ARCHITECTURAL TREATMENT, ETC. THE CONTRACTOR SHALL VERIFY DIMENSIONS AND RESOLVE DISCREPANCIES OR CONFLICTS PRIOR TO CONSTRUCTION.

APPLICABLE CODES AND STANDARDS

Table with 2 columns: Code and Description. Includes entries for Building Code, ACI 318-11, ACI 530-11, RCSC, AISC, AISC 360, AISI S100, ASCE 7-10, ASTM, AWS A2.4, AWS D1.1, AWS D1.3, AWS D1.4, AWS D1.8.

ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BUILDING CODE

STRUCTURAL DESIGN DATA

Table with 2 columns: FLOOR LIVE LOAD and SNOW LOADS. Lists various load types and their corresponding values in PSF.

SNOW LOADS: SNOW LOADING AND SNOW DRIFT LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (SECTION 1608).

Table with 2 columns: Ground Snow Load and Importance Factor. Lists values for Pg, Is, Ce, Ct, and Flat-Roof Snow Load.

WIND LOADS: WIND PRESSURE SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (SECTION 1609). Includes details for Basic Wind Speed, Risk Category, Exposure Category, Internal Pressure Coefficient, and Component Cladding.

SEISMIC LOADS: SEISMIC LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. Includes Risk Category and Importance Factor.

Table with 2 columns: Seismic Design Category and Mapped Spectral Acceleration Parameters. Lists values for Site Class, Ss, S1, Fa, Sds, and Sd1.

SEISMIC DESIGN IS EXEMPT DUE TO SDC A

Table with 2 columns: Wind Drift and Story Drift. Lists values for H/400 and H/400.

BUILDING ELEMENTS INCLUDING EXTERIOR CLADDING; STAIRS, ELEVATORS, AND MISCELLANEOUS METALS; MECHANICAL/ELECTRICAL/PLUMBING SYSTEM SUPPORTS; INTERIOR METAL STUD FRAMING; AND ANY OTHER ELEMENTS AS REQUIRED BY THE BUILDING CODE SHALL BE DESIGNED TO ACCOMMODATE THE PRIMARY STRUCTURE STORY DRIFTS WITH ANY APPLICABLE ELEMENT-SPECIFIC MODIFICATIONS PER CHAPTER 13 OF ASCE 7.

FOUNDATIONS AND SLAB ON GRADE

REFER TO THE FINAL GEOTECHNICAL REPORT PREPARED BY GEI CONSULTANTS INC, DATED NOVEMBER 12, 2018 FOR ALL GEOTECHNICAL REQUIREMENTS AND ANTICIPATED CONDITIONS AT AND BELOW GRADE. REFER TO THE CONTRACT DRAWINGS AND SPECIFICATIONS FOR EXCAVATION, BACKFILL, STRUCTURAL FILL, BASE MATERIAL, AND COMPACTION REQUIREMENTS.

FOUNDATIONS SHALL CONSIST OF SPREAD FOOTING WITH A 4500 PSF ALLOWABLE BEARING CAPACITY. PLACE FOOTINGS ON SUITABLE UNDISTURBED SOILS OR STRUCTURAL FILL APPROVED BY THE GEOTECHNICAL ENGINEER. WHERE SUITABLE UNDISTURBED SOILS ARE NOT FOUND AT THE SPECIFIED FOOTING ELEVATION, OVER-EXCAVATE TO THE DEPTHS REQUIRED BY THE GEOTECHNICAL ENGINEER AND REPLACE MATERIALS WITH STRUCTURAL FILL, LEAN CONCRETE, OR PROVIDE OTHER PREPARATION AS DIRECTED BY THE GEOTECHNICAL ENGINEER TO ACHIEVE THE REQUIRED ALLOWABLE BEARING CAPACITY.

CONCRETE SLAB ON GRADE SHALL BE PLACED ON PREPARED BASE COURSE PER THE GEOTECHNICAL ENGINEER. BASE COURSE SHALL CONSIST OF CRUSHED AGGREGATE COMPACTED TO THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. REFER TO THE PLANS AND DETAILS FOR OTHER REQUIREMENTS. REFER TO THE ARCHITECT AND GEOTECHNICAL ENGINEER FOR VAPOR BARRIER AND UNDER-SLAB DRAINAGE REQUIREMENTS.

ALL FILL PLACED TO SUPPORT SLABS ON GRADE, BEHIND PERMANENT WALLS, AND AROUND ALL DRAINS SHALL CONSIST OF WELL GRADED, GRANULAR MATERIAL PER THE SPECIFICATIONS. SOILS USED FOR STRUCTURAL FILL SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. STRUCTURAL FILL SHALL BE PLACED ON SOUND NATIVE MATERIAL. PROOF-ROLL CUT AREAS THAT PROVIDE SUPPORT FOR PERMANENT STRUCTURES. AREAS THAT ARE EXCESSIVELY YIELDING, AS DETERMINED BY THE CONTINUOUS OBSERVATION OF THE GEOTECHNICAL ENGINEER, SHALL BE OVEREXCAVATED AND REPLACED WITH STRUCTURAL FILL.

THE DESIGN PRESSURES FOR SUBGRADE WALLS ARE BASED ON A "DRAINED" CONDITION. SEE CIVIL AND MECHANICAL DRAWINGS FOR SUBGRADE DRAINAGE SYSTEM. SEE GEOTECHNICAL REPORT FOR COMPACTION REQUIREMENTS AT SUBGRADE WALLS. SUBGRADE WALLS AND SUPPORTING SLABS SHALL HAVE ATTAINED THEIR FULL CONCRETE STRENGTH BEFORE PLACING ANY BACKFILL. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACES FOR WALLS IF BACKFILL IS PLACED BEFORE WALLS AND SLABS ACHIEVE FULL CONCRETE STRENGTH.

CONCRETE

MIXING, BATCHING, TRANSPORTING, PLACING, AND CURING OF ALL CONCRETE, AND SELECTION OF CONCRETE MATERIALS, SHALL CONFORM TO ACI 301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE," EXCEPT AS NOTED BELOW. PROPORTIONS OF AGGREGATE TO CEMENTITIOUS PASTE SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX THAT CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER.

MIX DESIGNS LISTED BELOW SHALL BE SUBMITTED TO THE ARCHITECT AND APPROVED PRIOR TO USE. SELECTION OF CONCRETE MIX PROPORTIONS SHALL BE IN ACCORDANCE WITH ACI 301. MIX PROPORTIONS SHALL MEET OR EXCEED THE REQUIREMENTS WHERE THE CONCRETE IS USED.

THE CEMENTITIOUS MATERIAL CONTENT SHALL BE ADEQUATE FOR THE SPECIFIED REQUIREMENTS FOR STRENGTH, WATER-CEMENT RATIO, DURABILITY, AND FINISHABILITY.

ALL CONCRETE USED IN HORIZONTAL SURFACES EXPOSED TO THE WEATHER SHALL CONTAIN AN ACCEPTABLE ADMIXTURE TO PRODUCE AIR-ENTRAINED CONCRETE WITH TOTAL AIR CONTENT AS NOTED IN THE CONCRETE MIX SPECIFICATION.

CONCRETE MIX SPECIFICATION TABLE

Table with 6 columns: Location, fc (PSI), Test Days, W/C Ratio, Max Aggregate Size, Air Content Percentage. Lists various concrete locations and their mix specifications.

IN ADDITION TO ANY CAMBER NOTED IN THE STRUCTURAL DRAWINGS, CONCRETE FORMWORK SHALL BE CAMBERED TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD. CAMBERS OF LESS THAN 1/8 INCH MAY BE NEGLECTED.

FLOOR SLABS SHALL BE CONSTRUCTED TO THE THICKNESS SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FLOOR TOLERANCES. THE CONTRACTOR SHALL INCLUDE THE QUANTITIES OF THE ADDED CONCRETE DUE TO THE STEEL DECK DEFLECTION.

ALL CONSTRUCTION JOINTS IN SLABS, BEAMS, AND WALLS SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS. INTENTIONAL ROUGHENING WHERE NOTED AS AN ALTERNATE SHALL OCCUR BY SAND BLASTING OR CHIPPING HAMMER TO EXPOSE THE AGGREGATE EMBEDDED IN THE PREVIOUS POUR SO THAT IT PROTRUDES AND HAS A MINIMUM AMPLITUDE OF 1/4 INCH. ALL CONSTRUCTION JOINT SURFACES SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, THE JOINTS SHALL BE WETTED AND STANDING WATER REMOVED.

ALL CONSTRUCTION JOINTS FOR SLABS ON DECK SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS.

ALL CONSTRUCTION, CONTROL, AND ISOLATION JOINTS FOR SLABS ON GRADE SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAILS.

THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF JOINTS TO THE ENGINEER FOR ACCEPTANCE BEFORE STARTING CONSTRUCTION. PROVIDE JOINTS AT LOCATIONS SPECIFICALLY NOTED ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS.

WHERE REQUIRED BY THE ARCHITECT, CONCRETE TOPPING SLABS WITH A MINIMUM THICKNESS OF 3 INCHES AND MAXIMUM THICKNESS OF 7 INCHES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE ARCHITECTURAL DETAILS. TYPICAL TOPPING SLABS SHALL BE REINFORCED WITH WELDED WIRE FABRIC, WWF 4 X 4 - W4.0 X W4.0 UNLESS NOTED OTHERWISE. TOPPING SHALL HAVE CONTROL JOINTS AT 10'-0" ON CENTER MAXIMUM EACH WAY UNLESS NOTED OTHERWISE.

REFER TO THE ARCHITECTURAL DRAWINGS FOR ALL CONCRETE DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DEVELOP DETAILED SLAB EDGE AND CONCRETE OUTLINE DRAWINGS BASED ON THE ARCHITECTURAL, STRUCTURAL, AND MEP DRAWINGS. THE DETAILED EDGE AND OUTLINE DRAWINGS SHALL BE SUBMITTED FOR REVIEW. SUBMITTED DRAWINGS SHALL CONTAIN ALL CONCRETE CURBS, FORM OUTLINES, AND EMBEDDED ITEMS. DIMENSIONS AND OUTLINES DEVELOPED BY THE CONTRACTOR MAY VARY FROM THOSE SHOWN BY THE ARCHITECT AND ENGINEER AS NECESSARY BASED ON THE DEPENDENCY ON ADJACENT MATERIALS THAT ARE DETERMINED BY THE CONTRACTOR AND/OR SUPPLIER (EXTERIOR CLADDING, ELEVATOR EQUIPMENT, FINAL MEP SHAFT SIZES, ETC.). CONCRETE OUTLINES SHALL BE ADJUSTED AS NECESSARY TO ACCOUNT FOR CONSTRUCTION METHODS AND FOR SLAB SHRINKAGE AND THE CONCRETE OUTLINE DEVELOPED BY THE CONTRACTOR SHALL NOT MATERIALLY ALTER THE DESIGN INTENT SHOWN ON THE STRUCTURAL DRAWINGS.

EXCEPT AS DETAILED ON STRUCTURAL DRAWINGS, NO CONCRETE FOOTINGS, BEAMS, OR GIRDERS SHALL BE SLEEVED FOR PIPING OR DUCTS, UNLESS APPROVED BY THE ENGINEER.

ANCHORAGE TO HARDENED CONCRETE SHALL INCLUDE MECHANICAL AND ADHESIVE ANCHORS OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS. HOLES SHALL BE DRILLED AND CLEANED AND ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTRUCTIONS AND AN APPROVED ICC-ES REPORT. INSPECTION AND TESTING SHALL BE PROVIDED IN ACCORDANCE WITH THE GENERAL NOTES AND THE APPROVED ICC-ES REPORT.

WHERE THE ANCHOR TYPE IS SPECIFIED ON THE DRAWINGS, SUBSTITUTION FOR A DIFFERENT TYPE OF ANCHORAGE (INCLUDING SUBSTITUTING FOR CAST-IN-PLACE ANCHORAGE) SHALL NOT BE PERMITTED WITHOUT PRIOR CONSENT OF THE ENGINEER.

UNLESS NOTED OTHERWISE, ANCHORS SHALL BE ASTM A36 THREADED RODS OR ASTM A615, GRADE 60 REINFORCING STEEL DOWELS.

WHEN EMBEDMENT IS NOTED ON THE DRAWINGS, THE ANCHOR EFFECTIVE EMBEDMENT DEPTH SHALL EQUAL OR EXCEED THE NOTED EMBEDMENT DEPTH. WHERE NO EMBEDMENT IS NOTED ON THE DRAWINGS, THE MINIMUM EFFECTIVE ANCHOR EMBEDMENT DEPTH SHALL BE 6.5 x ANCHOR DIAMETERS, MINIMUM DISTANCE TO THE NEAREST CONCRETE EDGE SHALL BE 12 x ANCHOR DIAMETERS, AND MINIMUM ANCHOR SPACING SHALL BE 8 x ANCHOR DIAMETERS. STAINLESS STEEL ANCHORS SHALL BE USED AT ALL EXTERIOR LOCATIONS AND WHERE SPECIFICALLY INDICATED ON THE DRAWINGS. NO STEEL REINFORCEMENT SHALL BE CUT TO INSTALL ANCHORS.

HOLES SHALL BE DRILLED WITH ROTARY IMPACT HAMMER OR EQUIVALENT METHOD TO PRODUCE A HOLE WITH A ROUGH INSIDE SURFACE. CORE DRILLING HOLES IS NOT PERMITTED. THE ADHESIVE SHALL BE MIXED, APPLIED, AND CURED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS IN THE ICC - ES REPORT. ALL PLACEMENT AND CURING SHALL BE CONDUCTED WITH CONCRETE AND AIR TEMPERATURES ABOVE 50 DEGREES FAHRENHEIT. ADHESIVE SHALL BE APPLIED ONLY TO CLEAN, DRY CONCRETE. POSITIVE PROTECTION SHALL BE PROVIDED SO THAT ANCHORS ARE NOT DISTURBED DURING THE CURING PERIOD. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT OR AN INJECTABLE ADHESIVE MATCHING THE ADJACENT CONCRETE COMPRESSIVE STRENGTH. NOTIFY THE STRUCTURAL ENGINEER OF DEFECTIVE OR ABANDONED HOLES IN WALLS AND COLUMNS. THESE ELEMENTS MAY REQUIRE NON-SHRINK GROUT WITH A COMPRESSIVE MODULUS OF ELASTICITY MATCHING THAT OF THE ADJACENT CONCRETE.

ELECTRICAL CONDUIT SHALL BE RIGID STEEL CONDUIT OR FLEXIBLE PLASTIC CONDUIT. ALUMINUM CONDUIT IS PROHIBITED.

FOR CONDUIT PLACED IN CONCRETE FLAT SLABS OR SLABS THAT ARE PART OF A CONCRETE SLAB AND BEAM SYSTEM, CONDUIT SHALL HAVE A MAXIMUM OUTSIDE DIAMETER OF 1/6 TIMES THE SLAB THICKNESS AND SHALL BE EMBEDDED WITHIN THE MIDDLE THIRD OF THE SLAB DEPTH. MINIMUM CLEAR DISTANCE BETWEEN CONDUITS SHALL BE THREE TIMES THE CONDUIT DIAMETER.

FOR CONDUIT PLACED IN SLABS ON STEEL DECKING, CONDUIT SHALL ONLY RUN IN THE STEEL DECK FLUTES PER THE TYPICAL CONDUIT IN SLAB ON STEEL DECK DETAIL. CONDUIT SHALL NOT BE PLACED ABOVE DECK FLUTES IN EITHER DIRECTION. THIS CONDUIT SHALL ROUTE UNDER THE SLAB ON METAL DECK OR AN ALTERNATIVE LOCATION WHICH SHALL BE COORDINATED BY THE CONTRACTOR WITH THE ARCHITECT AND OTHER TRADES.

CONDUIT SHALL BE FIRMLY CHAIRED AND TIED TO PREVENT DISPLACEMENT DURING POURING. PLACE #4 AT 12 INCHES ADDITIONAL REINFORCING ABOVE CONDUIT RUNNING ABOVE STEEL DECK FLUTES AND ABOVE AND BELOW CONDUIT IN CONCRETE SLABS, PERPENDICULAR TO THE CONDUIT. THE ADDED REINFORCING SHALL EXTEND 1'-0" PAST THE CONDUIT ON BOTH SIDES.

POLYSTYRENE OR RIGID INSULATION PLACED BELOW CONCRETE SLABS SHALL CONSIST OF RIGID CELLULAR POLYSTYRENE CONFORMING TO ASTM D6817. POLYSTYRENE SHALL HAVE A MINIMUM COMPRESSIVE RESISTANCE OF 3.6 PSI AT 1 PERCENT DEFORMATION UNLESS NOTED OTHERWISE. SECURE POLYSTYRENE IN PLACE PER THE MANUFACTURER'S RECOMMENDATIONS. THE BLOCKS OF POLYSTYRENE SHALL BE PLACED TO OFFSET JOINTS 24 INCHES BETWEEN THE ADJACENT LAYERS.

AT THE CONTRACTOR'S OPTION, IN LIEU OF POLYSTYRENE CONFORMING TO ASTM D6817, PROVIDE POLYSTYRENE CONFORMING TO ASTM C578 TYPE XIV RATED FOR 40 PSI COMPRESSIVE RESISTANCE AT 10 PERCENT DEFORMATION WITH A MINIMUM THICKNESS OF 2 INCHES PER LAYER.

GROUT SHALL BE AN APPROVED NON-SHRINK CEMENTITIOUS GROUT CONTAINING NATURAL AGGREGATES DELIVERED TO THE JOB SITE IN FACTORY PREPACKAGED CONTAINERS REQUIRING ONLY THE ADDITION OF WATER. THE MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE AT LEAST 1,000 PSI HIGHER THAN THE SUPPORTING CONCRETE STRENGTH, UNLESS NOTED OTHERWISE. GROUT SHALL BE MIXED, APPLIED, AND CURED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. FOR GROUTING UNDER BASE PLATES, GROUT SHALL BE PROPORTIONED AS A FLOWABLE MIX. WHEN A FLOWABLE MIX DOES NOT PROVIDE THE REQUIRED STRENGTH OR WHEN A MINIMUM STRENGTH OF 10,000 PSI IS REQUIRED, AN EPOXY GROUT SHALL BE USED.

REINFORCING STEEL

REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT." BARS SHALL BE SUPPORTED ON CHAIRS IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE. ALL REINFORCING BARS SHALL BE SECURELY TIED IN PLACE WITH #16 GAGE MIN ANNEALED BLACK WIRE. EPOXY-COATED REINFORCING BARS SHALL BE TIED WITH NYLON, EPOXY-OR PLASTIC-COATED TIE WIRE OR OTHER ACCEPTABLE MATERIALS. SHOP DRAWINGS, INCLUDING PLACING PLANS AND ELEVATIONS SHALL BE SUBMITTED TO, AND REVIEWED BY, THE ARCHITECT/ENGINEER BEFORE STARTING FABRICATION.

REINFORCING BARS SHALL BE LAP SPLICED FOR TENSION (Lst) UNLESS NOTED OTHERWISE ON THE DRAWINGS. #14 AND #18 BARS SHALL BE SPLICED USING MECHANICAL COUPLINGS INCLUDING SPLICES WITH SMALLER BARS. #14 AND #18 BARS SHALL NOT BE LAP SPLICED. AT THE CONTRACTOR'S OPTION, MECHANICAL COUPLINGS MAY BE USED FOR ANY BAR SIZE, PROVIDED A CURRENT ICC - ES REPORT DEMONSTRATES THAT THE PRODUCT CAN ACHIEVE A MINIMUM

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES

GOVERNMENT OF THE DISTRICT OF COLUMBIA PERMIT OPERATIONS DIVISION PLANS APPROVED

Permit No. PD1900028 Date 05/21/19

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MEP ENGINEER: Sully & Associates International 3040 Williams Drive, Suite 600 Fairfax, VA 22031 Phone: 703-691-2115

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LANDSCAPE ARCHITECT: Landscape Architecture Bureau 714 7th Street, SE Washington, DC 20003 Phone: 202-543-6550



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Issues / Revisions

Table with 2 columns: Date and Description. Includes entries for 11/20/2018 Schematic Design Submission and 01/17/2019 Foundation to Grade Permit.

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Ward 1 STFH & PSH 2500 14th Street NW Washington DC 20009

CQA#2018038

Drawing Title

GENERAL NOTES

Table with 2 columns: Scale and Drawn By. Includes entries for 12" = 1'-0" and CS.

12/07/2018 Checked BY

S0101

TENSILE STRENGTH OF 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE BAR. NO REINFORCING BARS SHALL BE SPICED BY WELDING. SPLICE DEVICES SHALL HAVE A CURRENT ICC - ES REPORT THAT SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. HEADED BARS OR TERMINATORS SHALL BE PROVIDED WHERE INDICATED ON THE DRAWINGS OR AT THE CONTRACTOR'S OPTION FOR CONGESTED AREAS OF REINFORCEMENT ANCHORAGE SUBJECT TO THE ENGINEER'S APPROVAL. HEADED BARS OR TERMINATORS SHALL MEET THE REQUIREMENTS OF ACI 318 AND ASTM A970, AND HAVE A CURRENT ICC - ES REPORT.

WELDED WIRE FABRIC (WWF) SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A1064. LAP EDGES AND ENDS OF FABRIC A MINIMUM OF ONE MESH SPACING PLUS 2 INCHES, BUT NOT LESS THAN 6 INCHES. WELDED WIRE FABRIC SHALL BE SUPPORTED ON CHAIRS IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE. MINIMUM WWF IS 6x6-W2.9xW2.9, UNLESS NOTED OTHERWISE

WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TO PLATES, ANGLES, ETC, IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS CERTIFIED WELDERS USING E9018 OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.4.

EPOXY COATED REINFORCING BARS SUPPORTED FROM FORMWORK SHALL REST ON COATED WIRE BAR SUPPORTS, OR ON BAR SUPPORTS MADE OF DIELECTRIC MATERIAL OR OTHER ACCEPTABLE MATERIALS. WIRE BAR SUPPORTS SHALL BE COATED WITH DIELECTRIC MATERIAL FOR A MINIMUM DISTANCE OF 2 INCHES FROM THE POINT OF CONTACT WITH THE EPOXY COATED REINFORCING BARS. REINFORCING BARS USED AS SUPPORT BARS SHALL BE EPOXY COATED.

MINIMUM CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS NOTED OTHERWISE, SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST EARTH: ALL BAR SIZES:	3	INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER: #6 BAR OR LARGER: #5 BAR OR SMALLER:	2 1 1/2	INCHES
OTHER CONCRETE: SLABS: #14 AND #18 BARS: #11 BARS AND SMALLER: TOP BARS: BOTTOM BARS:	1 1/2 3/4 1	INCHES INCH
WALLS: #14 AND #18 BARS: #11 BARS AND SMALLER:	1 1/2 1	INCHES INCH
BEAMS AND COLUMNS, TIES, STIRRUPS, SPIRRALS, ALL BAR SIZES:	1 1/2	INCHES

SPECIFIED CONCRETE COVER SHALL BE MAINTAINED TO ALL REINFORCEMENT AT CONCRETE REVEALS AND INSETS. SHOP DRAWINGS SHOWING CONCRETE REVEALS AND OTHER INSETS SHALL BE SUBMITTED FOR REVIEW. .

STRUCTURAL STEEL

ALL STEEL SHALL CONFORM TO THE FOLLOWING:

W-SHAPES	ASTM A992, ASTM A913,	Fy=50 KSI Fy=50 KSI
ALL ANGLES AND CHANNELS UNLESS NOTED OTHERWISE	ASTM A36,	Fy=36 KSI
SQUARE OR RECTANGULAR STRUCTURAL TUBE (HSS) ROUND STRUCTURAL TUBE (HSS)	ASTM A500, GRADE C ASTM A500, GRADE C	Fy=50 KSI Fy=46 KSI
STEEL PIPE DIAMETER LESS THAN OR EQUAL TO 12 INCHES	ASTM A53, TYPE E OR S GRADE B,	Fy=35 KSI
MATERIAL CALLED OUT ON PLANS AS (A36)	ASTM A36,	Fy=36 KSI
MATERIAL CALLED OUT ON PLANS AS (Fy=65 KSI)	ASTM A913,	Fy=65 KSI
ALL OTHER STEEL UNLESS NOTED OTHERWISE	ASTM A572, ASTM A588,	Fy=50 KSI Fy=50 KSI

ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATION. SHOP DRAWINGS SHALL BE SUBMITTED AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE COMMENCING FABRICATION. CONNECTION DESIGN SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATIONS AND THE BUILDING CODE. CONNECTIONS SHALL BE CAPABLE OF RESISTING ALL LOADS LISTED ON THE DRAWINGS. MEMBERS SHOWN ON THE DRAWINGS HAVE NOT BEEN SIZED FOR LOCAL EFFECTS AT CONNECTIONS AND THE CONNECTION DESIGN SHALL PROVIDE STIFFENER PLATES, WEB DOUBLE PLATES, FLANGE CONTINUITY PLATES, ETC, AS REQUIRED.

UNLESS SPECIFICALLY DETAILED COMPETELY ON THE STRUCTURAL DRAWINGS, CONNECTION DETAILS SHOWN ILLUSTRATES THE GENERAL DESIGN AND DETAILING CRITERIA. THE CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, WHO SHALL DESIGN AND COMPLETE THE CONNECTIONS. SUBMIT STAMPED CALCULATIONS TO THE ARCHITECT FOR REVIEW AND APPROVAL PROR TO FABRICATION.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS.

ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED, AND INSPECTED IN ACCORDANCE WITH THE RCSC. BOLTS IN ALL CONNECTIONS MAY BE SNUG TIGHT UNLESS SPECIFICALLY CALLED OUT AS SLIP CRITICAL (SC).

WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT, THE CONTRACTOR MAY INSTALL PER THE CRITERIA FOR SNUG-TIGHT BOLTS. SLIP-CRITICAL CONNECTIONS SHALL USE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. ALL ASTM A307 BOLTS SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF-LOCKING NUTS. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS NOTED OTHERWISE.

PROVIDE MINIMUM A TWO-BOLT CONNECTION USING 7/8-INCH-DIAMETER A325 BOLTS IN SINGLE SHEAR.

PROVIDE MINIMUM 3/8" THICK PLATE UNLESS OTHERWISE NOTED.

ALL ENDS OF BEARING CONNECTIONS SHALL BE MILLED TO COMPLETE TRUE BEARING.

ALL STRUCTURAL STEEL NOT RECEIVING SPRAY-ON FIREPROOFING SHALL BE PRIMED. PARTS OF STRUCTURAL STEEL LEFT UNPAINTED BECAUSE OF FIELD WELDING OR BOLTING SHALL RECEIVE A FIELD APPLICATION OF METAL PROTECTION. ALL STEEL ELEMENTS IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES

THE USE OF SHOP AND FIELD WELDS. ALL PARTIAL JOINT PENETRATION GROOVE WELD SIZES SHOWN ON THE DRAWINGS REFER TO EFFECTIVE THROAT THICKNESS. ALL WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH PER AWS D1.1 (MINIMUM 70 KSI). LOW HYDROGEN SMAW ELECTRODES SHALL BE USED WITHIN 4 HOURS OF OPENING THEIR HERMETICALLY SEALED CONTAINERS, OR SHALL BE REBAKED PER AWS D1.1, SECTION 4.5. ELECTRODES SHALL BE REBAKED NO MORE THAN ONE TIME, AND ELECTRODES THAT HAVE BEEN WET SHALL NOT BE USED.

ALL WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1. ALL WELDING PARAMETERS SHALL BE WITHIN THE ELECTRODE MANUFACTURER'S RECOMMENDATIONS. WELDING PROCEDURES SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WPS SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.

ALL COMPLETE JOINT PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED UPON COMPLETION OF THE CONNECTION, EXCEPT PLATE LESS THAN OR EQUAL TO 1/4 INCH THICK SHALL BE MAGNETIC PARTICLE TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF THE ENGINEER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE JOINT PREPARATIONS AND WELDING PROCEDURES THAT INCLUDE, BUT ARE NOT LIMITED TO: REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPEs, SURFACE ROUGHNESS VALUES, AND TAPERS AND TRANSITIONS OF UNEQUAL PARTS.

REFER TO ARCHITECTURAL PLANS FOR MINIMUM HOURLY VALUES OF STEEL FIRE PROTECTION FOR DETERMINING THE THICKNESS OF SPRAY APPLIED FIREPROOFING. THE STRUCTURAL FRAME CONSISTS OF COLUMNS AND GIRDERS, BEAMS, TRUSSES, AND SPANDRELS HAVING DIRECT CONNECTIONS TO THE COLUMNS AND BRACING MEMBERS DESIGNED TO CARRY GRAVITY LOADS. FLOOR OR ROOF MEMBERS THAT HAVE NO CONNECTION TO COLUMNS SHALL BE CONSIDERED SECONDARY MEMBERS. ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 WITH CLASS 2A THREADS, UNLESS NOTED OTHERWISE. FURNISH ANCHOR RODS PREFABRICATED WITH MATCHING DOUBLE HEAVY HEX NUTS JAMMED AT THE END EMBEDDED IN CONCRETE. FURNISH HARDENED PLATE WASHERS, LOCK WASHERS, AND MATCHING HEAVY HEX NUTS FOR SECURING THE BASE PLATE TO THE ANCHOR RODS. HOOKED ANCHOR RODS SHALL NOT BE USED EXCEPT WHERE NOTED. ANCHOR ROD NUTS SHALL BE INSTALLED TO A SNUG-TIGHT CONDITION. NO HEATING OR BENDING OF THE ANCHOR RODS IS PERMITTED. HOLES IN THE BASE MATERIAL SHALL NOT BE ENLARGED BY BURNING.

MASONRY

HOLLOW MASONRY UNITS USED IN NON-LOAD BEARING WALLS SHALL CONFORM TO ASTM C129, LOAD-BEARING MASONRY UNITS SHALL CONFORM TO C90.

ALL MASONRY UNITS SHALL BE HOLLOW MEDIUM WEIGHT, WITH MINIMUM BLOCK COMPRESSIVE STRENGTH OF 1,900 PSI UNLESS OTHERWISE NOTED. STANDARD WEIGHT OF UNITS SHALL BE 30 PSF FOR 6" UNITS; 38 PSF FOR 8" UNITS, 47 PSF FOR 10" UNITS AND 55 PSF FOR 12" UNITS. WEIGHT TOLERANCE FOR ALL TYPES OF MASONRY UNITS SHALL BE LESS THAN 3 PSF HIGHER OR LOWER THAN THE STANDARD WEIGHT.

MASONRY UNITS CONTAINING REINFORCEMENT SHALL BE FILLED SOLID WITH CONCRETE GROUT. GROUT MIX SHALL CONTAIN PORTLAND CEMENT, AGGREGATE, AND A GROUT- ENHANCING SHRINKAGE-COMPENSATING ADDITIVE. MINIMUM GROUT COMPRESSIVE STRENGTH BASED ON 28-DAY TESTS SHALL BE 2,000 PSI. GROUT SHALL BE VIBRATED WHILE PLACING TO ENSURE THAT CELLS ARE COMPLETELY FILLED. SUBMIT GROUT MIXES TO ARCHITECT FOR REVIEW BEFORE COMMENCING MASONRY CONSTRUCTION. ALL UNITS SHALL BE LAID IN RUNNING BOND WITH HEAD JOINTS. MASONRY MINIMUM DESIGN STRENGTH fm SHALL BE 1,500 PSI.

PROVIDE GALVANIZED HORIZONTAL JOINT REINFORCEMENT AT 16" ON CENTER OVER FULL HEIGHT OF ALL WALLS. JOINT REINFORCEMENT SHALL BE CONTINUOUS AROUND BUILDING WITH PREFORMED CORNER AND TEE UNITS. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN FIRST UN-REINFORCED COURSE ABOVE AND BELOW EACH OPENING, WITH LENGTH EQUAL TO 36" GREATER THAN THE OPENING WIDTH UNLESS OTHER NOTED.

AT SPANDREL COLUMN AND BEAM LOCATIONS, ANCHOR MASONRY WALLS TO STEEL MEMBERS WITH 3/16"Ø ADJUSTABLE TIES AT A SPACING OF 16" MAXIMUM ALONG THE MEMBER UNLESS OTHERWISE NOTED. GROUT CELLS OF MASONRY SOLID AROUND ALL MASONRY ANCHORS.

BRACE TOP OF ALL PARTITION MASONRY WALLS TO STRUCTURE PER TYPICAL DETAILS. MAINTAINING MIN. 1" SOFT JOINT FOR INDEPENDENT VERTICAL MOVEMENT OF THE STRUCTURE UNLESS OTHERWISE NOTED.

FILL ALL HOLLOW MASONRY UNITS BELOW GRADE WITH GROUT OR MORTAR.

PLACE GROUT IN LIFTS OF NO MORE THAN 4' IN HEIGHT.

REINFORCE ALL EXTERIOR OR BEARING WALLS WITH #6@24" ON CENTER UNLESS OTHERWISE NOTED. IF BAR SPACING WOULD FALL WITHIN AN OPENING, RELOCATE BARS TO T NEAREST JAMB. IN ADDITION TO STANDARD WALL REINFORCING, PROVIDE 1-#6 VERTICAL AT EACH SIDE OF ANY OPENING. IF 2 BARS ARE TO BE PLACED IN A JAMB CELL, PLACE AT INSIDE AND OUTSIDE FACES OF WALL. IF 3 OR MORE BARS ARE TO BE PLACED IN A JAMB CELL, PLACE 2 BARS PER CELL AT INSIDE AND OUTSIDE FACES. PROVIDE HORIZONTAL JOINT REINFORCING AT 8" ON CENTER EXTENDING OUT 4' FROM FACE OF JAMB. DO NOT INTERRUPT JAMB BARS AT LINTELS.

ALL REINFORCING SPLICES SHALL BE A MINIMUM OF 48 BAR DIAMETERS.

LINTELS

COORDINATE WITH ARCHITECTURAL DRAWINGS FOR TYPES OF LINTEL REQUIRED.

PROVIDE MASONRY BOND BEAM UNIT WITH THICKNESS EQUAL TO MIN. WIDTH OF WALL.

PROVIDE ONE STEEL ANGLE FOR EACH 4" OF WALL THICKNESS FOR THE FOLLOWING OPENINGS UNLESS OTHERWISE NOTED OR SHOWN ON CONTRACT DOCUMENTS:

OPENINGS UP TO 3'-4"	L3-1/2X3-1/2X5/16 (LLV)
OPENINGS 3'-5" TO 5'-0"	L4X3-1/2X5/16 (LLV)
OPENINGS 5'-1" TO 6'-0"	L5X3-1/2X5/16 (LLV)

FOR OPENINGS 6'-1" UP TO 8'-6", PROVIDE W8 X 15 WITH 5/16" SUSPENDED PLATE BY 5/16" PLATE HANGERS @ 16" O.C. UNLESS OTHERWISE NOTED. SEE TYPICAL DETAIL.

FOR OPENINGS 8'-7" UP TO 11'-0", PROVIDE W8 X 24 WITH 5/16" SUSPENDED PLATE AND 5/16" PLATE HANGERS @ 16" o.c. UNLESS OTHERWISE NOTED.

FOR OPENINGS 11'-1" UP TO 13'-6", PROVIDE W16x31 WITH 5/16" SUSPENDED PLATE AND 5/16" PLATE HANGERS @ 16" O.C. UNLESS OTHERWISE NOTED.

FOR OPENINGS 13'-7" UP TO 16'-6", PROVIDE W16x40 WITH 5/16" SUSPENDED PLATE AND 5/16" PLATE HANGERS @ 16" O.C. UNLESS OTHERWISE NOTED.

FOR OPENINGS 16'-7" UP TO 20'-0", PROVIDE W16x45 WITH 5/16" SUSPENDED PLATE AND 5/16" PLATE HANGERS @ 16" O.C. UNLESS OTHERWISE NOTED.

FOR OPENINGS 20'-1" UP TO 24'-6", PROVIDE W16x50 WITH 5/16" SUSPENDED PLATE AND 5/16" PLATE HANGERS @ 16" O.C. UNLESS OTHERWISE NOTED.

MINIMUM BEARING AT EACH END SHALL BE 6" FOR STEEL AND 8" FOR PRECAST AND CAST-IN-PLACE LINTELS. PROVIDE 2-1/2"Ø x 8" ANCHOR BOLTS INTO GROUTED CELL AT EACH END OF STEEL BEAM LINTELS.

PROVIDE SHOP PRIMER FOR ALL INTERIOR STEEL LINTELS AND PROVIDE HOT-DIPPED GALVANIZING FOR ALL STEEL EXTERIOR LINTELS.

LIGHT GAGE STEEL

MINIMUM YIELD STRENGTH OF THE LIGHT GAGE FRAMING COMPONENTS SHALL BE 33KSI FOR 18 GA OR LIGHTER AND 50KSI FOR 16 GA AND HEAVIER.

MAXIMUM DEFLECTION OF WALL STUDS BACKUP FOR BRICKS SHALL BE L/600 OR 3/8", WHICHEVER IS LESS. ALL OTHERS SHALL BE L/360 . L IS THE STUD LENGTH BETWEEN ITS SUPPORTS.

STUD BACKUP SYSTEM SHALL BE DESIGNED AS A FLOOR TO FLOOR SYSTEM WITHOUT KICKERS.

LIGHT GAGE STEEL FRAMING AND THEIR CONNECTIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN JURISDICTION OF WHERE THE PROJECT IS LOCATED TO CONFORM WITH THE APPLICABLE BUILDING CODES AND PROJECT REQUIREMENTS. THE CONTRACTOR SHALL PROVIDE LIGHT GAGE SHOP DRAWINGS AND SIGNED AND SEALED CALCULATIONS TO THE ARCHITECT FOR APPROVAL BEFORE COMMENCING WORK. ANY SIZES INDICATED IN THE DRAWING ARE FOR INFORMATION ONLY.

SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS SHOWING MEMBER LAYOUT, SIZE AND SPACING. PROVIDE CONNECTION DETAILS SUCH AS BRACING, SPLICES, ACCESSORIES, AND OTHER DETAILS THAT MAY BE REQUIRED FOR PROPER INSTALLATION.

LIGHT GAGE STUD CAPACITY SHALL BE REDUCED APPROPRIATLY AT RATED LOADING BEARING WALLS. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR FIRE RATING

THE SPECIALTY ENGINEER SHALL VISIT THE JOBSITE DURING CONSTRUCTION TO VERIFY CONFORMANCE TO THE SHOP DRAWINGS AT COMPLETION OF THE PROJECT PROVIDE A LETTER CERTIFYING THE CONSTRUCTION IS IN ACCORDANCE WITH THE DESIGN.

PROVIDE MINIMUM G60 GALVANIZED COATING FOR ALL LIGHT GAGE STUDS EXCEPT BACKUP STUDS FOR BRICKS, WHICH SHOULD HAVE G90 AND MINIMUM 18 GAGE.

ALL LOAA BEARING STEEL STUDS SHALL HAVE MINIMUM 1.625 INCH FLANGE AND FLANGE RETURN UP.

WELD STUDS USING E60XX ELECTRODES BY CERTIFIED WELDERS. FOR MECHANICAL FASTENERS USE ONLY POLYMER FINISHES, CHROMIUM PLATED, STAINLESS STEEL OR HOT DIP GALVANIZED UNITS.

STEEL DECK

COMPOSITE SLABS OF CONCRETE AND STEEL DECK SHALL BE CONSTRUCTED IN ACCORDANCE WITH ASCE 3.

COMPOSITE FLOOR DECK SHALL BE 20 GAGE 2 INCH DEEP VERSA-DEK MANUFACTURED BY NEW MILLENNIUM. FLOOR DECKS SHALL BE SHORED AS REQUIRED BY THE MANUFACTURER DURING CONSTRUCTION. DECKING SHALL BE GALVANIZED. MINIMUM REQUIREMENTS FOR FASTENING SHALL BE 5/8 INCH DIAMETER PUDDLE WELDS AT 8 INCHES ON CENTER AT STRUCTURAL STEEL SUPPORTS AND #10 TEK SCREWS AT 8 INCHES ON CENTER AT STUD WALLS WITH 1-#10 TEK SCREWS SIDELAP FASTENER AT 24 INCHES ON CENTER.

STAIRS, ELEVATORS, MISCELLANEOUS METALS AND MEP SUPPORTS

UNLESS SHOWN AND DETAILED IN THE STRUCTURAL DRAWINGS, ALL STAIRS ARE TO CONSIST OF A PRE-FABRICATED AND PRE-ENGINEERED STAIR, LANDING, AND RAILING SYSTEM DESIGNED BY THE CONTRACTOR OR STAIR SUPPLIER. SEE THE ARCHITECT FOR STAIR SYSTEM LAYOUT, DIMENSIONS, AND CONFIGURATION OF RISE AND RUN. THE CONTRACTOR SHALL BE RESPONSIBLE TO DESIGN AND PROVIDE THE STAIR SYSTEM INCLUDING ALL CONNECTIONS AND SECONDARY SUPPORT FRAMING. THE STAIRS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES. UNDER THE SERVICE LEVEL STORY DRIFTS, ALL STAIRS MUST REMAIN FUNCTIONAL. UNDER THE DESIGN STORY DRIFTS ALL EGRESS STAIRS MUST REMAIN FUNCTIONAL AND ALL OTHER STAIRS MUST REMAIN CONNECTED TO THE BUILDING.

ALL ELEVATOR MACHINE BEAMS, HOIST BEAMS, SILLS, DOOR SUPPORTS, AND RAILS AND THEIR CONNECTIONS TO THE STRUCTURE ARE TO BE DESIGNED BY THE ELEVATOR MANUFACTURER. THE ELEVATORS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

ALL FRAMING AND CONNECTIONS DESIGNED BY THE CONTRACTOR SHALL NOT CAUSE TORSIONAL LOADS OR WEAK AXIS BENDING TO THE STRUCTURAL MEMBERS. THE CONTRACTOR'S DESIGN SHALL VERIFY THAT THE CONNECTIONS DO NOT RESULT IN ADVERSE LOCAL CONNECTION STRESSES OCCURRING WITHIN THE PRIMARY STRUCTURE. SUBMIT CALCULATIONS STAMPED BY A STRUCTURAL ENGINEER LICENSED TO PERFORM THE WORK IN THE JURISDICTION WHERE THE PROJECT IS LOCATED AND SHOP DRAWINGS INDICATING IMPOSED LOADS ON THE PRIMARY STRUCTURE.

THE CONTRACTOR SHALL DESIGN AND SUPPLY ALL ADDITIONAL MISCELLANEOUS METALS AND SYSTEM SUPPORT COMPONENTS THAT ARE NECESSARY TO SUPPORT ALL MECHANICAL, ELECTRICAL (TELECOM, AUDIO VISUAL, ETC), AND PLUMBING/FIRE-PROTECTION SYSTEMS. SUCH METALS AND SUPPORT COMPONENTS AND THEIR CONNECTIONS SHALL BE PROVIDED AS NECESSARY TO DIRECTLY AND CONCENTRICALLY IMPOSE LOADS ON THE PRIMARY STRUCTURE. THE CONNECTIONS TO THE PRIMARY STRUCTURE ARE SUBJECT TO THE REQUIREMENTS OF THE MISCELLANEOUS METALS SECTION ABOVE. THESE SYSTEMS MAY BE SUPPORTED DIRECTLY FROM STEEL ROOF AND COMPOSITE FLOOR/ROOF SLABS SUBJECT TO THE FOLLOWING LIMITATIONS: 250 POUNDS MAY HANG FROM COMPOSITE SLAB ON DECK, 50 POUNDS MAY HANG FROM STEEL ROOF DECK. LOADS SHALL BE LOCATED NO CLOSER THAN 5 FEET FROM ANY ADJACENT HANGING LOAD, AND THE CONTRACTOR SHALL COORDINATE THE SUPPORT AND HANGING LOADS FROM ALL BUILDING SYSTEMS. THE MECHANICAL/ELECTRICAL/PLUMBING SYSTEM SUPPORTS SHALL ACCOMMODATE LATERAL MOVEMENTS BETWEEN ADJACENT FLOORS AS DEFINED IN THE STORY DRIFT SECTION OF THESE NOTES.

SHOP DRAWINGS

REFER TO PROJECT SPECIFICATIONS FOR GENERAL SHOP DRAWINGS REQUIREMENTS.

THE CONTRACTOR SHALL SUBMIT CONCRETE WALL ELEVATION DRAWINGS OF AT LEAST 1/8" = 1'-0" SCALE INDICATING LOCATIONS OF CONNECTION EMBEDMENTS AND WALL OPENINGS IN REVIEW PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL COORDINATE WITH REINFORCEMENT DRAWINGS.

DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD AND SHALL BE VERIFIED BY THE CONTRACTOR.

THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED, AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWINGS SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED

SHOP DRAWINGS FOR DEFERRED SUBMITTALS THAT ARE DEFINED AS DESIGN-BUILD COMPONENTS IN THE CONSTRUCTION DOCUMENTS SHALL INCLUDE THE DESIGNING PROFESSIONAL ENGINEER'S STAMP FOR THE JURISDICTION WHERE THE PROJECT IS LOCATED AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. DESIGN CALCULATIONS SHALL BE INCLUDED IN THE SUBMITTAL.

GOVERNMENT OF THE DISTRICT OF COLUMBIA
PERMIT OPERATIONS DIVISION
PLANS APPROVED

Permit No. FD1900028 Date 05/21/19

These plans are conditionally approved as submitted or noted during plan review and are subject to field inspection. Approved plans must be kept on site and are needed for all inspections. No changes or modifications to these plans. Changes require a revision permit with a valid contractor's license. Permits are required for trade work, e.g. Electrical or Plumbing

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Issues / Revisions

11/20/2018 Schematic Design Submission

01/17/2019 Foundation to Grade Permit

CQA#2018038

Drawing Title

Scale 12" = 1'-0"

Drawn By CS

Date 12/07/2018

Checked BY

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES

S0102

SPECIAL INSPECTION

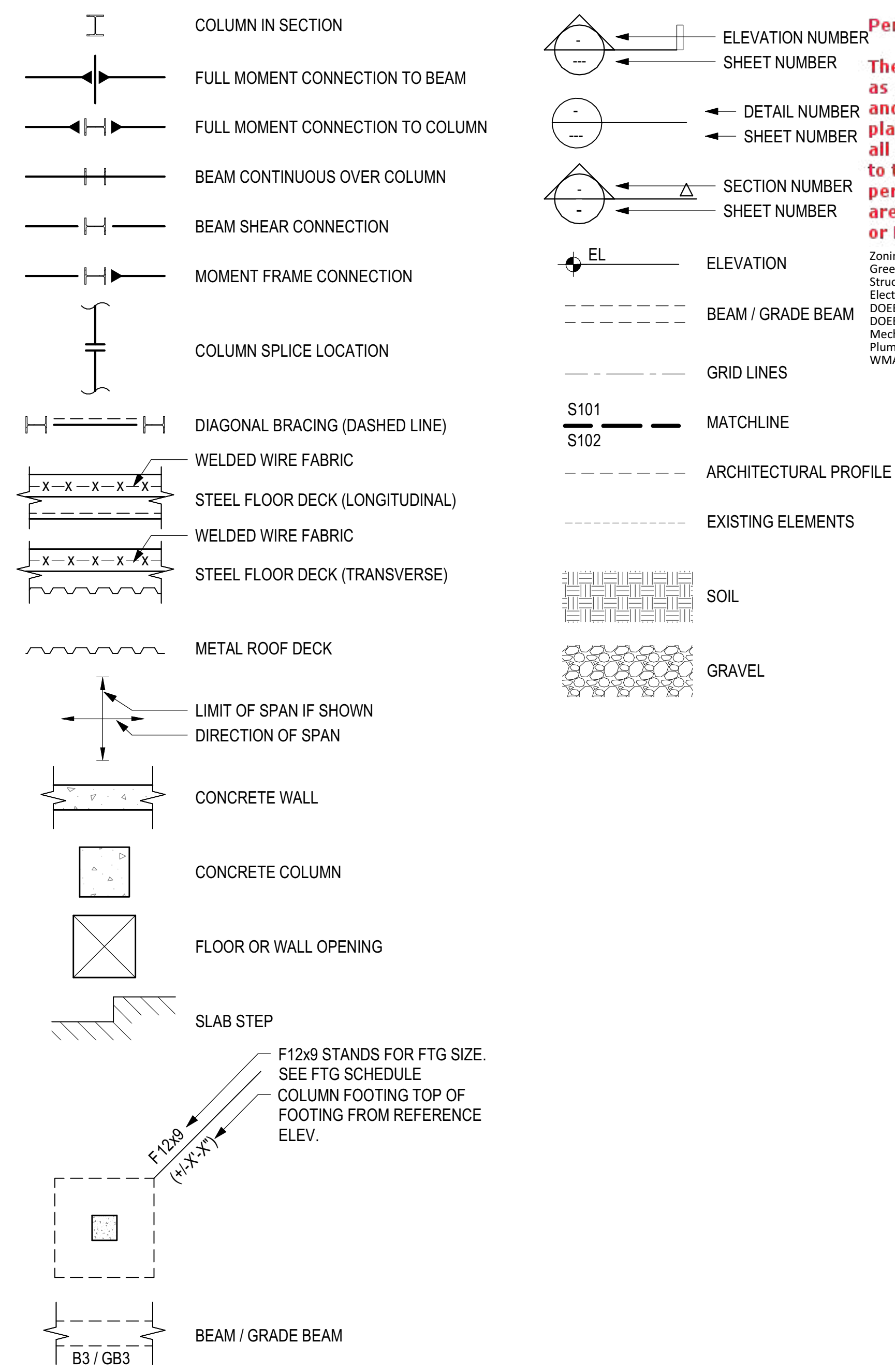
THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION AND TESTING PER IBC SECTION 1705. THIS WORK SHALL BE PERFORMED BY A SPECIAL INSPECTOR CERTIFIED BY THE DISTRICT OF COLUMBIA TO PERFORM THE TYPES OF INSPECTIONS AND TESTS SPECIFIED. THE FREQUENCY OF INSPECTIONS AND TESTING SHALL BE AS OUTLINED IN THE IBC TABLE ITEMS LISTED BELOW. DEFICIENCIES SHALL BE REPORTED DAILY TO THE CONTRACTOR. SUMMARY REPORTS SHALL BE DISTRIBUTED WEEKLY TO THE OWNER, ARCHITECT, CONTRACTOR, BUILDING OFFICIAL, AND STRUCTURAL ENGINEER. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION AND TESTING.

ITEM	DESCRIPTION (REFER TO IBC SECTION 1705)	IBC TABLE REQUIREMENTS
STRUCTURAL STEEL AND WELDING	STRUCTURAL STEEL THAT IS PART OF THE STRUCTURE.	SECTION 1705.2
HIGH STRENGTH BOLTING	SEE SPECIFICATIONS FOR PROCEDURES FOR INSPECTION AND TESTING.	SECTION 1705.2
CONCRETE	CONCRETE THAT IS PART OF THE STRUCTURE.	TABLE 1705.3, ITEMS 5, 6, 7, 8
ANCHORS CAST IN CONCRETE	ANCHOR BOLTS, HEADED STUDS.	TABLE 1705.3, ITEM 3
ANCHORS INSTALLED IN HARDENED CONCRETE	INSTALLATION OF MECHANICAL AND ADHESIVE ANCHORS IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND THE REQUIREMENTS OF THE ICC-ES REPORT FOR THE PRODUCT INSTALLED.	TABLE 1705.3, ITEM 4
REINFORCING STEEL	A. PLACEMENT OF REINFORCING B. SPLICING OF REINFORCING BY BUTT WELDING, EXOTHERMIC WELDING PROCESS, OR THREADED COUPLERS.	TABLE 1705.3, ITEM 1, TABLE 1705.3, ITEM 1, 2
LIGHT GAUGE FRAMING	MATERIAL AND CONNECTIONS	
SOILS		SECTION 1705.6

ABBREVIATIONS

&	AND	FL	FLOOR	R	RADIUS
@	AT	FLG	FLANGE	REF	REFERENCE
#	NUMBER, POUND	FP	FIREPROOF; FULL PENETRATION	REINF	REINFORCING; REINFORCEMENT
		FRMG	FRAMING	REQD	REQUIRED
AB	ANCHOR BOLT	FS	FAR SIDE	REQT	REQUIREMENT
ADDL	ADDITIONAL	FT	FOOT; FEET		
ADJ	ADJACENT	FTG	FOOTING		
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	GA	GAGE, GAUGE	SC	SLIP CRITICAL
AGGR	AGGREGATE	GALV	GALVANIZED	SCHED	SCHEDULE, SCHEDULED
ALT	ALTERNATE	GB	GRADE BEAM	SECT	SECTION
ALUM	ALUMINUM	GL	GLUED LAMINATED	SEOR	STRUCTURAL ENGINEER OF RECORD
APA	AMERICAN PLYWOOD ASSOCIATION	GRND	GROUND	SHT	SHEET
APPD	APPROVED			SIM	SIMILAR
APPROX	APPROXIMATE	H	HORIZONTAL	SLBB	SHORT LEGS BACK-TO-BACK
AR	ANCHOR RODS	HA	HANGER ABOVE	SOG	SLAB ON GRADE
ARCH	ARCHITECTURAL; ARCHITECT	HB	HANGER BELOW	SP	SPIRAL
ASSY	ASSEMBLY	HDG	HOT DIP GALVANIZIE	SPC	SPACING
		HEF	HORIZONTAL EACH FACE	SPEC	SPECIFICATION
		HGR	HANGER	SQ	SQUARE
B/L	BUILDING LINE	HORIZ	HORIZONTAL	SSL	SHORT SLOTTED HOLES
BAL	BALANCE	HP	HIGH POINT	STD	STANDARD
BD	BOARD	HS	HIGH STRENGTH	STIFF	STIFFENER
BF	BRACED FRAME	HT	HEIGHT	STIRR	STIRRUP
BLDG	BUILDING			STL	STEEL
BLK	BLOCK; BLOCKING	ID	INSIDE DIAMETER	STR	STRAIGHT
BM	BEAM	IN	INCH	STRUC	STRUCTURAL
BMU	BRICK MASONRY UNIT	INCL	INCLUDE	SUPT	SUPPORT
BOS	BOTTOM OF STEEL; BOSOM (WELD)	INFO	INFORMATION	SW	SHORT WAY
BOT	BOTOM	INSUL	INSULATION	SYM	SYMMETRICAL
BRCG	BRACING	INT	INTERIOR		
BRG	BEARING	JST	JOIST	T&B	TOP AND BOTTOM
BRKT	BRACKET	JT	JOINT	T&G	TONGUE AND GROOVE
BSMT	BASEMENT			T&R	TREAD AND RISER
BU	BUILT-UP			TEMP	TEMPERATURE; TEMPORARY
		K	KIPS (1,000 POUNDS)	THK	THICK; THICKNESS
C	CAMBER	KO	KNOCK-OUT	TOC	TOP OF CURB, TOP OF CONCRETE
CA	COLUMN ABOVE	KSI	KIPS PER SQUARE INCH	TOF	TOP OF FOOTING
CANT	CANTILEVER			TOS	TOP OF STEEL
CB	COLUMN BELOW	L	ANGLE	TOSL	TOP OF SLAB
CC	CENTER TO CENTER	LB, #	POUND	TOW	TOP OF WALL
CG	CENTER OF GRAVITY	LF	LINEAR FOOT	TRANS	TRANSVERSE
CIP	CAST-IN-PLACE	LIN	LINEAR, LINTEL	TYP	TYPICAL
CJ	CONTROL JOINT	LL	LIVE LOAD		
CJP	COMPLETE JOINT PENETRATION	LLBB	LONG LEGS BACK-TO-BACK	UL	UNDERWRITER'S LABORATORY
		LLH	LONG LEG HORIZONTAL	UNO,UON	UNLESS NOTED OTHERWISE
CH	CHANNEL	LLV	LONG LEG VERTICAL		
CL	CENTER LINE	LOC	LOCATION; LOCATE	V, VERT	VERTICAL
CLR	CLEARANCE; CLEAR	LONGIT	LONGITUDINAL	VG	VERTICAL GRAIN
CMU	CONCRETE MASONRY UNIT	LP	LOW POINT		
COL	COLUMN	LSL	LONG SLOTTED (HOLE)	W/	WITH
COMP	COMPRESSION	LTWT	LIGHT WEIGHT	W/O	WITHOUT
CONC	CONCRETE	LVL	LEVEL	WD	WOOD
CONFIG	CONFIGURATION	LW	LONG WAY	WF	WIDE FLANGE
CONN	CONNECTION; CONNECT	LWC	LIGHT WEIGHT CONCRETE	WH	WEEP HOLE
CONST	CONSTRUCTION			WP	WORK POINT
CONT	CONTINUE; CONTINUOUS	MAS	MASONRY	WT	WEIGHT
CONTR	CONTRACTOR	MATL	MATERIAL	WWF	WELDED WIRE FABRIC
COORD	COORDINATE; COORDINATION	MAX	MAXIMUM		
CORR	CORRUGATED	MECH	MECHANICAL	YD	YARD
CTR	CENTER	MEMEB	MEMBRANE		
CTSK	COUNTERSINK; COUNTERSUNK	MEP	MECHANICAL/ELECTRICAL/PLUMBING		
CU	CUBIC	MEZZ	MEZZANINE		
		MF	MOMENT FRAME		
d	PENNY (NAIL)	MFR	MANUFACTURE; MANUFACTURER		
db	NORMINAL BAR DIAMETER (INCHES)	MFRG	MANUFACTURING		
DBA	DEFORED BAR ANCHOR	MIN	MINIMUM		
DBL	DOUBLE	MISC	MISCELLANEOUS		
DEG, °	DEGREE	ML	MATCH LINE		
DIA, ø	DIAMETER	MO	MASONRY OPENING		
DIAG	DIAGONAL	MS	MECHANICAL SPLICE		
DIAPH	DIAPHRAGM				
DICA	DRILLED-IN CONCRETE ANCHOR	N-S	NORTH-SOUTH		
DIM	DIMENSION	NF	NEAR FACE		
DISC	DISCONTINUED; DISCONTINUOUS	NIC	NOT IN CONTRACT		
DL	DEAD LOAD	NO	NUMBER		
DN	DOWN	NTS	NOT TO SCALE		
DO	DITTO	NWC	NORMAL WEIGHT CONCRETE		
DWG	DRAWING				
DWL	DOWEL	OC	ON CENTER		
		OD	OUTSIDE DIAMETER		
E-W	EAST-WEST	OPNG	OPENING		
EA	EACH	OPP	OPPOSITE (HAND)		
EE	EACH END	OPT	OPTION; OPTIONAL		
EF	EACH FACE	OVS	OVERSIZED (HOLES)		
EJ	EXPANSION JOINT	OWJ	OPEN WEB JOIST		
EL	ELEVATION				
ELEC	ELECTRICAL	P	PIPE		
ELEV	ELEVATION	PAF	POWER ACTUATED FASTNER		
ELVR	ELEVATOR	PC	PRECAST		
EMBED	EMBEDDED	PCF	PONDS PER CUBIC FOOT		
ENGR	ENGINEER	PCP	PRECAST CONCRETE PANEL		
EOD	EDGE OF DECK	PEN	PENETRATION		
EQ	EQUAL; EARTHQUAKE	PERP	PERPENDICULAR		
EQUIP	EQUIPMENT	PJP	PARTIAL JOINT PENETRATION		
ES	EACH SIDE				
EQUIV	EQUIVALENT	PL	PLATE		
ETC	ET CETERA	P/L	PROPERTY LINE		
EW	EACH WAY	PLC	PLACE		
EX	EXISTING	PLF	POUNDS PER LINEAR FOOT		
EXIST	EXISTING	PLYWD	PLYWOOD		
EXT	EXTERIOR	PP	PRECAST PANEL		
EXTD	EXTEND, EXTENDED	PREFAB	PRE-FABRICATED		
		PSF	POUNDS PER SQUARE FOOT		
FD	FLOOR DRAIN	PSI	POUNDS PER SQUARE INCH		
FDN	FOUNDATION				
FF	FAR FACE; FINISH FLOOR				
FIN	FINISH				

LEGEND



GOVERNMENT OF THE DISTRICT OF COLUMBIA
PERMIT OPERATIONS DIVISION

PLANS APPROVED
Permit No. FD1900028 Date 05/21/19

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Zoning Review - Brian Green Review - Kris Structural Review - Electrical Review - DOEE SE-SW Review - DOEE GAR Review - Mechanical Review - Plumbing Review - WMATA Review -

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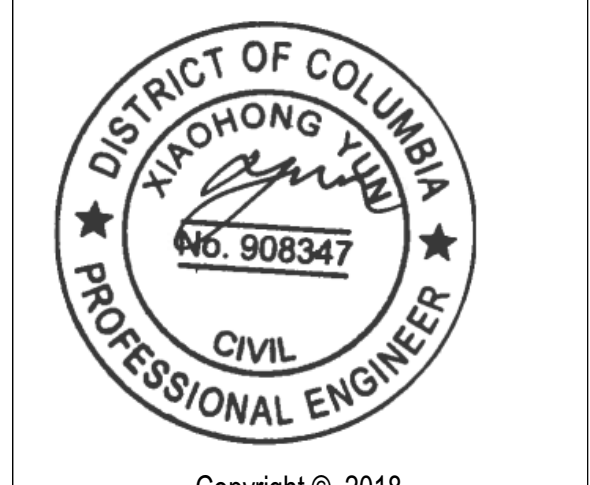
OWNER:
D.C. Department of General Services
1250 U Street, NW, 4th Floor
Washington, DC 20009
Phone: (202) 727-2800

STRUCTURAL ENGINEER:
Yun Associates, LLC
1775 K Street, NW, Suite 220
Washington, DC 20006
Phone: 202-649-3075

MEP ENGINEER:
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3040 Williams Drive, Suite 600
Fairfax, VA 22031
Phone: 703-691-2115

CIVIL ENGINEER:
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Phone: 202-638-4040 x255

LANDSCAPE ARCHITECT:
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714 7th Street, SE
Washington, DC 20003
Phone: 202-543-6550



Issues / Revisions

Date	Description
11/20/2018	Schematic Design Submission
01/17/2019	Foundation to Grade Permit

Ward 1 STFH & PSH
2500 14th Street NW
Washington DC 20009

CQA#2018038

Drawing Title

GENERAL NOTES

Scale	As indicated	Drawn By	CS
Date	12/07/2018	Checked	BY

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES

S0103

Department of Consumer & Regulatory Affairs
Statement of Special Inspections

Purpose: The purpose of this form is to capture the statement of special inspections and all professional stamps.

Instructions: All information that pertains to the applicant must be completed on all pages of this form.

Permit Numbers: FD1900028

Project Address: 2500 14th Street NW Washington DC 20009

This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Construction Code. It includes a Schedule of Special Inspections applicable to this project as well as the name of the Special Inspections Engineer(s) of Record, and the identity of other agents such as testing laboratories or agencies intended to be retained for conducting these inspections or tests.

The Special Inspections Engineer of Record (SIER) shall keep records of specified inspections, and shall furnish inspection reports to the Building Official, DCRA Inspector or Third Party Inspector, appropriate Registered Design Professionals (RDP), Owner and Contractor. All discrepancies shall be brought to the immediate attention of the Contractor and the Registered Design Professional in Responsible Charge for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and appropriate RDP(s). Interim reports and an activity/incident log shall be made available to the DCRA Inspector or Third Party Inspector according to the DCRA Special Inspections Manual.

All fees/costs related to the performance of Special Inspections shall be the responsibility of the Owner. Additionally, the undersigned (RDP or SIER) are only acknowledging that the items enumerated on the Schedule of Special Inspections are consistent with the required design elements, the applicable sections of the DCMR, and their area of expertise.

General Contractor: [Signature] **Company:** GCS, Inc. **License #:** 69010099

Owner or Owner's Agent: [Signature] **Date:** 3/7/19



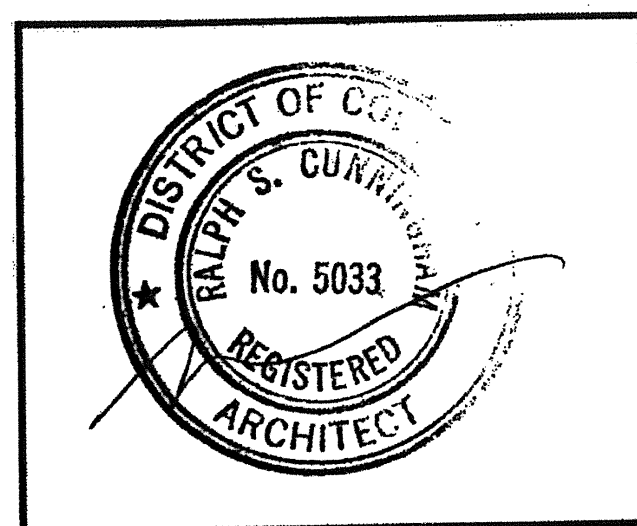
DCRA | 1100 4th Street SW, Washington, DC 20024 | 202.442.4400 | dcra.dc.gov

DCRA Statement of Special Inspections Form

Primary RDPs of Record:

Ralph Cunningham
 Full Name March 6, 2019
 Signature [Signature] Date
 Company or Firm Cunningham Quill Architects
202-337-0090
 Contact Number
rcunningham@cunninghamquill.com
 Email Address

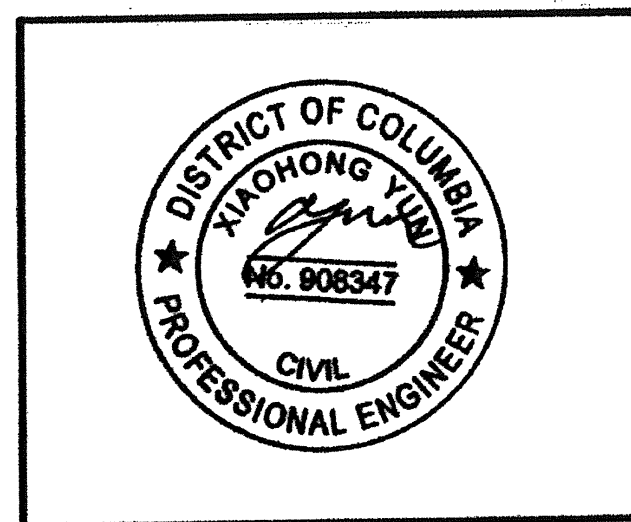
Place Stamp Here:



Structural Engineer of Record:

Xiaohong Yun
 Full Name March 6, 2019
 Signature [Signature] Date
 Company or Firm Yun Associates LLC
202-849-3075
 Contact Number
byun@yunassociates.com
 Email Address

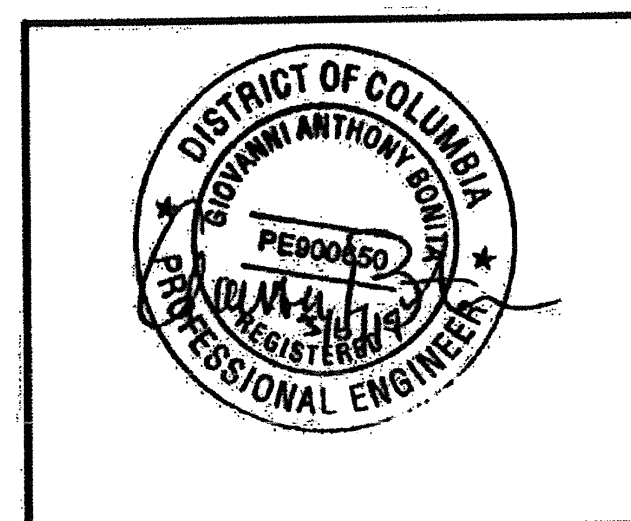
Place Stamp Here:



Special Inspections Engineer of Record RDP1:

Giovanni Bonita, PE, PG, PhD
 Full Name 3/5/19
 Signature [Signature] Date
 Company or Firm G&I Consultants, Inc.
202-828-9510
 Contact Number
gbonita@geiconsultants.com
 Email Address

Place Stamp Here:



*Note subsequent Special Inspections Engineer(s) of Record will be listed on schedule of Special Inspections.

DCRA | 1100 4th Street SW, Washington, DC 20024 | 202.442.4400 | dcra.dc.gov

DCRA Statement of Special Inspections Form

SECTION TO BE COMPLETED BY DCRA STAFF ONLY

Accepted by DCRA Building Official:

Full Name _____
 Signature _____ Date _____
 Title _____

DCRA | 1100 4th Street SW, Washington, DC 20024 | 202.442.4400 | dcra.dc.gov

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Zoning Review - Brie
 Green Review - Kris
 Structural Review -
 Electrical Review -
 DOEE SE-SW Review
 DOE GAR Review -
 Mechanical Review -
 Plumbing Review -
 WMATA Review - R

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Issues / Revisions

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CQA#2018038

Drawing Title

**STATEMENT OF
 SPECIAL INSPECTION**

Scale As indicated Drawn By CS
 Date 12/07/2018 Checked BY _____

**STRUCTURAL PLANS CERTIFIED AS PROVIDED IN
 SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES**

S0104

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Zoning Review - Brian Green Review - Kris Structural Review - Kris Electrical Review - ADOEE SE-SW Review DOEE GAR Review - Mechanical Review - Plumbing Review - WMATA Review - R

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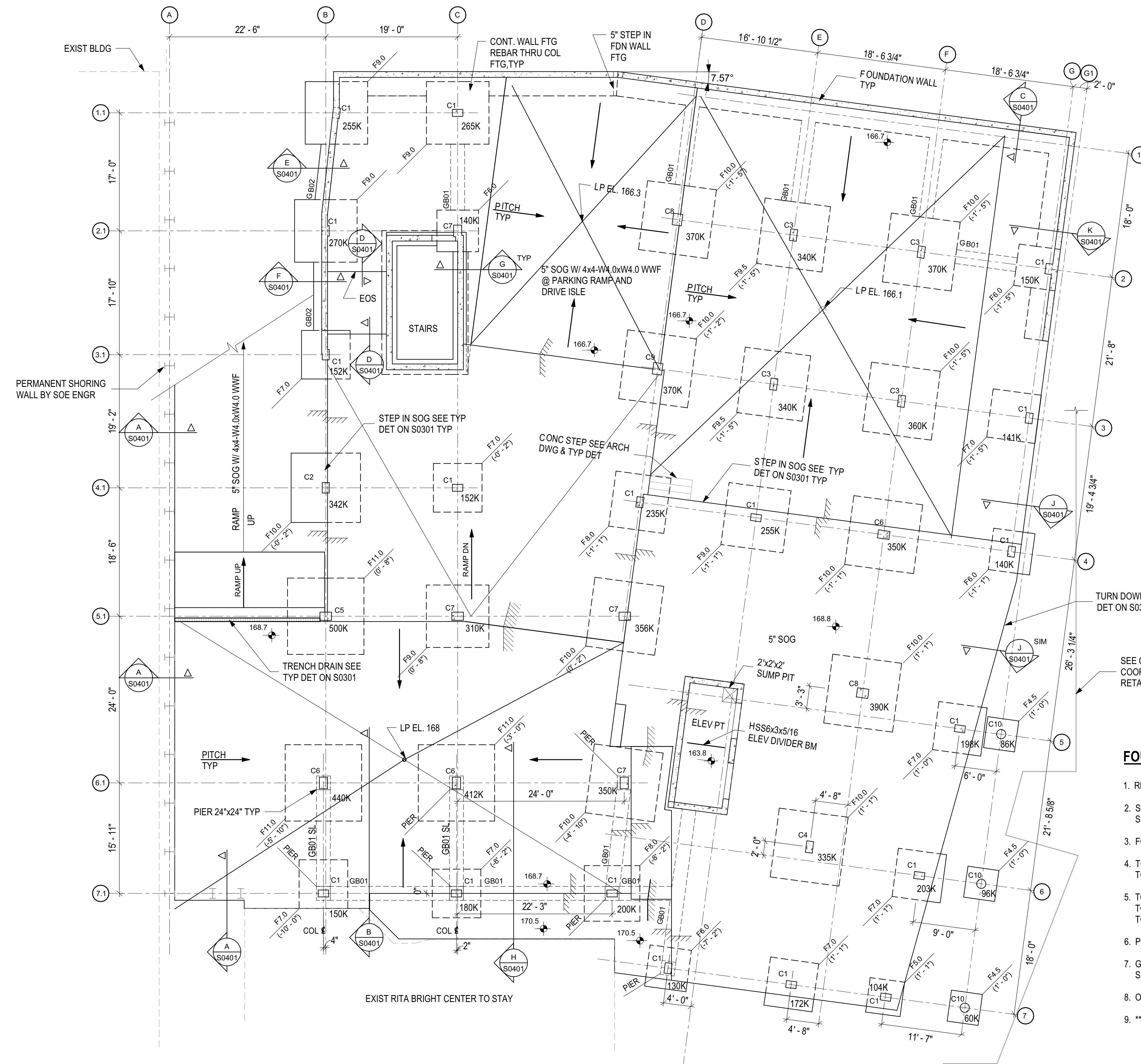
QQA#2018038

Drawing Title

FOUNDATION AND P1 FLOOR FRAMING PLAN

Scale: As indicated
 Date: 12/07/2018
 Drawn By: CS
 Checked By:

S0200



COLUMN SCHEDULE

Column	Section	Reinforcement
C1	12" x 18"	4#8
C2	12" x 18"	6#8
C3	12" x 20"	6#8
C4	12" x 20"	4#8
C5	14" x 20"	8#8
C6	14" x 20"	6#8
C7	14" x 20"	4#8
C8	16" x 20"	6#8
C9	16" x 20"	4#8
C10	16" DIA	6#8

FOOTING SCHEDULE

FOOTING	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F4.5	4'-6"	4'-6"	1'-6"	(7) #5 EW BOTT
F5.0	5'-0"	5'-0"	1'-6"	(7) #5 EW BOTT
F6.0	6'-0"	6'-0"	1'-8"	(6) #6 EW BOTT
F7.0	7'-0"	7'-0"	1'-8"	(7) #6 EW BOTT
F8.0	8'-0"	8'-0"	1'-10"	(7) #7 EW BOTT
F9.0	9'-0"	9'-0"	2'-0"	(8) #7 EW BOTT
F9.5	9'-6"	9'-6"	2'-0"	(8) #8 EW BOTT
F10.0	10'-0"	10'-0"	2'-3"	(8) #8 EW BOTT
F11.0	11'-0"	11'-0"	2'-3"	(9) #8 EW BOTT

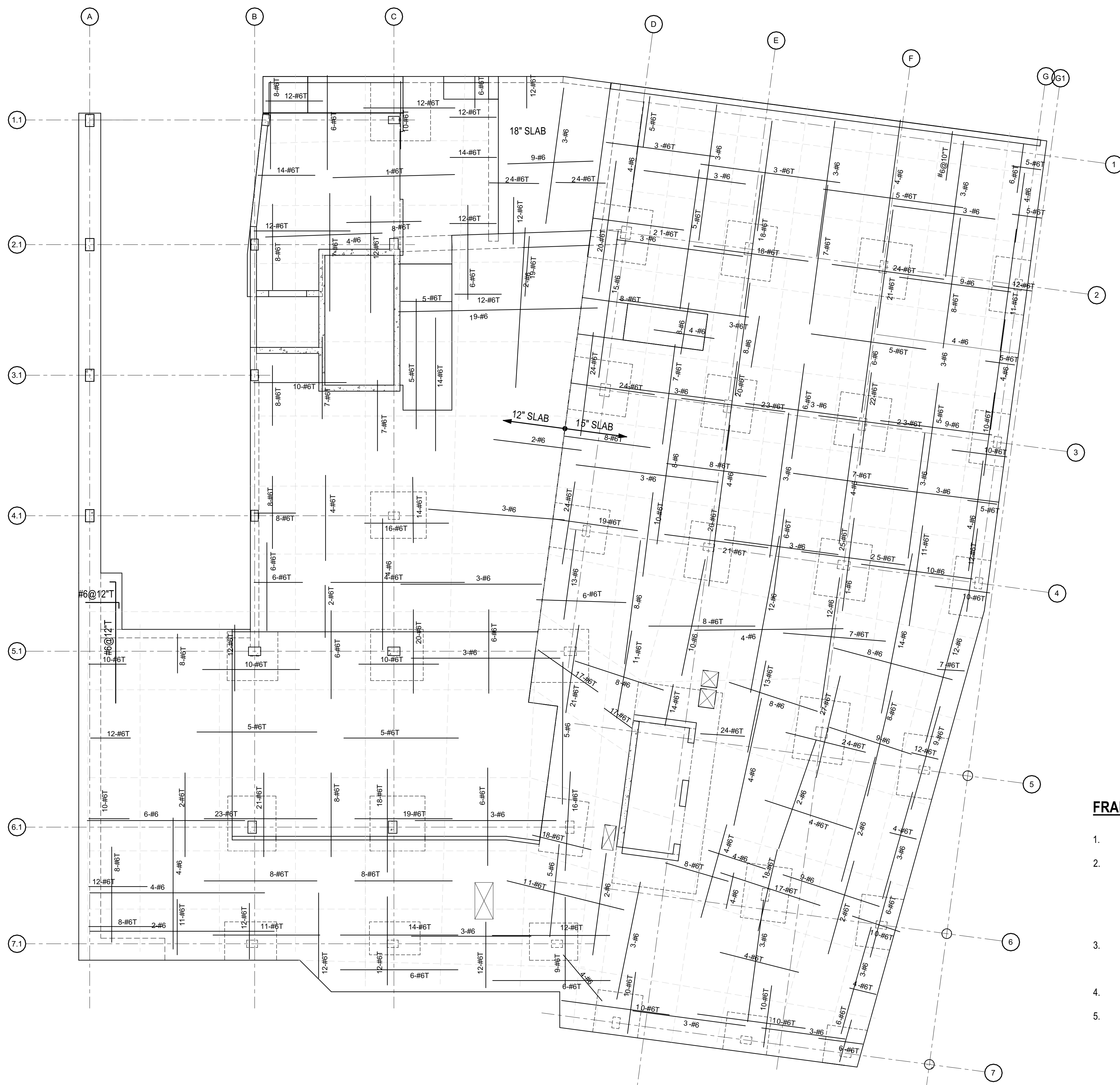
FOUNDATION PLAN NOTES:

- REFERENCE ELEVATION IS TOP OF SLAB ON GRADE ELEVATION AT 166.7' UNO ON PLAN.
- SLAB ON GRADE SHALL BE 5" NORMAL WEIGHT CONCRETE WITH 6x6 - W2.9xW2.9 WWF UNO ON PLAN. SEE TYPICAL DETAIL FOR SUBGRADE CONSTRUCTION.
- FOOTINGS SHALL BE CENTERED UNDER WALLS, COLUMNS, BEAMS AND PIERS UNO ON PLAN OR SECTIONS.
- TOP OF FOOTINGS SHALL BE 1'-0" BELOW REFERENCE ELEVATION UNO ON PLAN THUS (+/-X'-X") RELATIVE TO REFERENCE ELEVATION.
- TOP OF GRADE BEAM SHALL BE AT TOP OF FOOTINGS IT IS CONNECTED WITH UNO. WHERE PIERS OCCUR, TOP OF GRADE BEAM SHALL BE AT TOP OF PIERS IT IS CONNECTED WITH UNO. GRADE BEAM SLOPES IF TOP OF FOOTING OR PIERS ARE AT DIFFERENT ELEVATION.
- PIERS ARE 24"x24" REINFORCED WITH 8-#8 UNO.
- GB01 STANDS FOR GRADE BEAM OF 24" WIDE BY 18" DEEP WITH 3-#8 T&B AND 1-#8 SIDE BAR @ EF, SEE SECTIONS FOR GB02 REINFORCEMENT.
- ORIENTATION OF COLUMNS SHALL BE AS SHOWN TO SCALE ON PLAN.
- ***K NEAR COLUMN STANDS FOR TOTAL SERVICE LEVEL AXIAL LOADS AT TOP OF FTG.

FOUNDATION AND P1 FLOOR FRAMING PLAN

1/8" = 1'-0"

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES



GROUND FLOOR SLAB REBAR PLAN
1/8" = 1'-0"

FRAMING PLAN NOTES:

1. REFERENCE ELEVATION IS TOP OF GROUND FLOOR SLAB ELEVATION AT 179.3' UNO ON PLAN.
2. SLAB SHALL BE NORMAL WEIGHT CONCETE SLAB WITH 28 DAY COMPRESSIVE STRENGTH OF 5,000 PSI. 12" SLAB SHALL HAVE #6@12" EW CONTINUOUS BOTTOM MAT WITH 6" THICK DROP PANELS AND 15" SLAB SHALL HAVE #7@12" EW CONTINUOUS BOTTOM MAT WITH 8" THICK DROP PANELS. 18" SLAB SHALL HAVE #7@10" EW CONTINUOUS BOTTOM MAT. SEE PLAN FOR SLAB THICKNESS. REBAR OUTLAYERS SHALL RUN IN THE NORTH-SOUTH DIRECTION.
3. REBAR DESIGNATION WITHOUT "T" STANDS FOR BOOTOM BARS. ADDITIONAL REINFORCING SHOWN IN PLAN AS #6T STANDS FOR 8 NO. 6 BARS AT TOP SPACED WITHIN THE STRIP IT IS SHOWN.
4. SEE TYPICAL DETAILS FOR REBARS LAYOUT.
5. SEE BEAM SCHEDULES ON S0202 FOR BEAM REINFORCING.

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES

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Zoning Review - Brian
Green Review - Kris
Structural Review -
Electrical Review -
DOEE SE-SW Review -
DOEE GAR Review -
Mechanical Review -
Plumbing Review -
WMATA Review - R

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Drawing Title
GROUND FLOOR SLAB REBAR PLAN

Scale	As indicated	Drawn By	CS
Date	12/07/2018	Checked By	BY

S0201A

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Zoning Review - Brian Green Review - Kris Structural Review - Electrical Review - DOEE SE-SW Review - DOEE GAR Review - Mechanical Review - Plumbing Review - WMATA Review -

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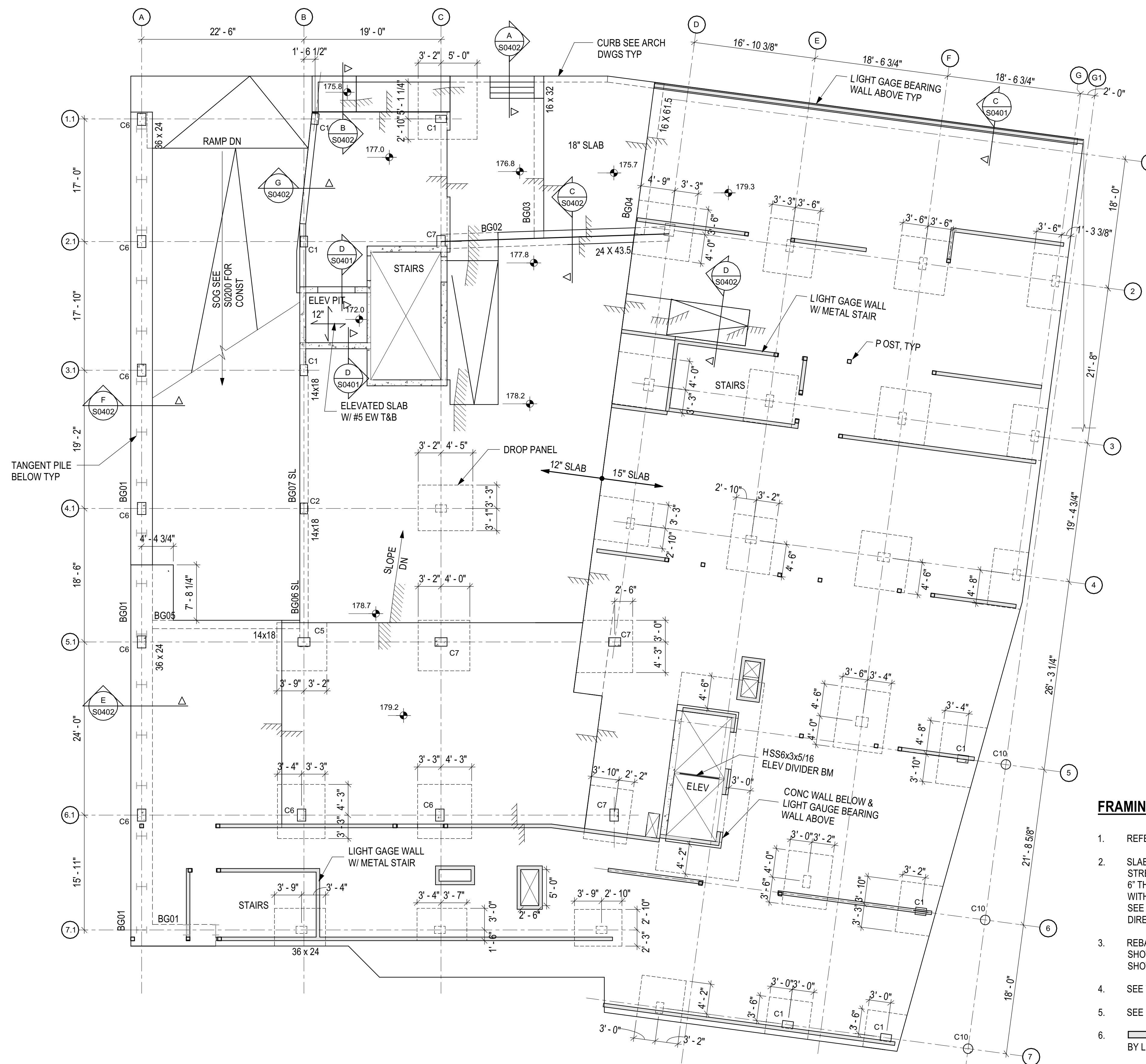
CQA#2018038

Drawing Title

GROUND FLOOR FRAMING PLAN

Scale	As indicated	Drawn By	CS
Date	12/07/2018	Checked By	BY

S0201



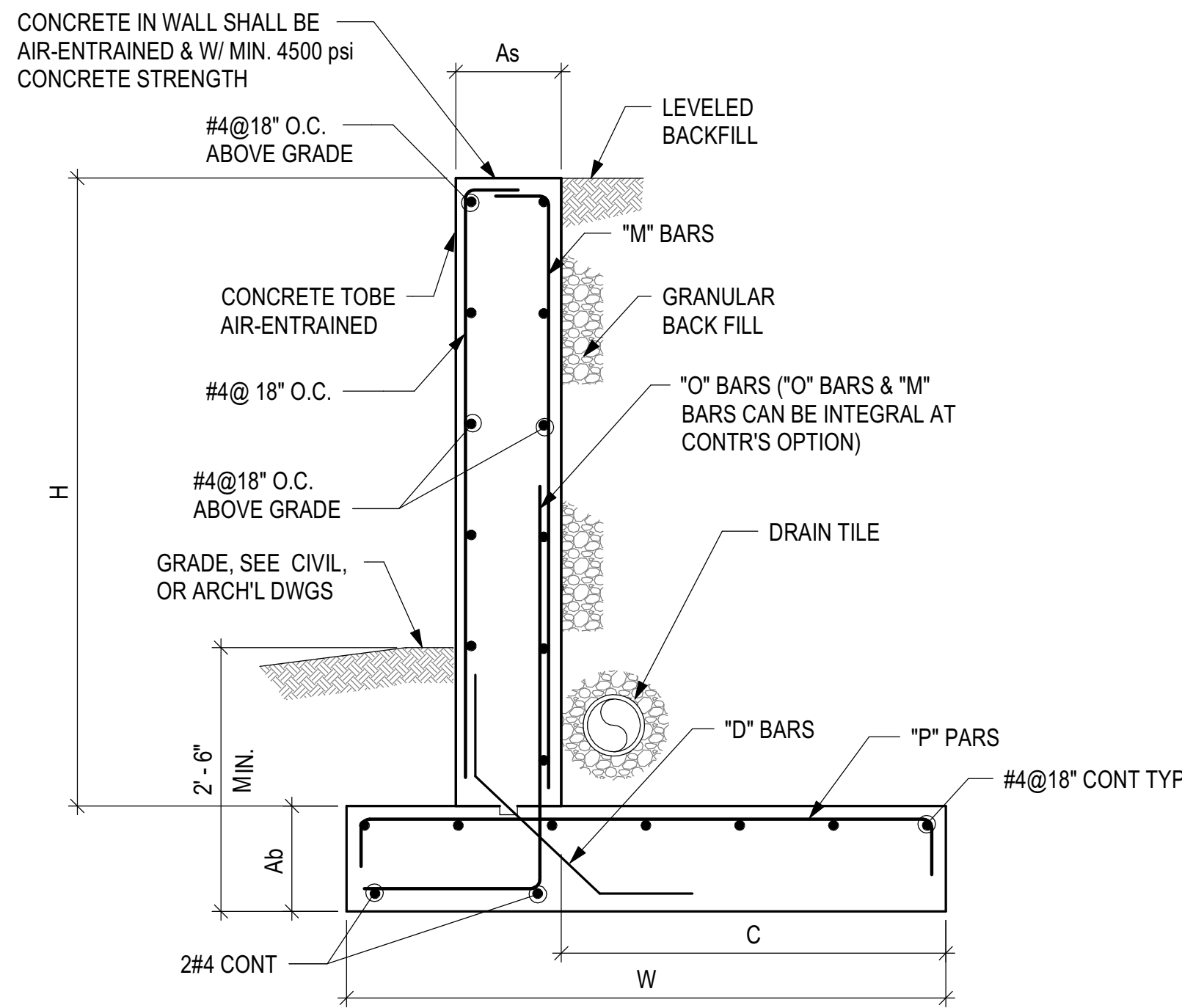
FRAMING PLAN NOTES:

- REFERENCE ELEVATION IS TOP OF GROUND FLOOR SLAB ELEVATION AT 179.3' UNO ON PLAN.
- SLAB SHALL BE NORMAL WEIGHT CONCRETE SLAB WITH 28 DAY COMPRESSIVE STRENGTH OF 5,000 PSI. 12" SLAB SHALL HAVE #6@12" EW CONTINUOUS BOTTOM MAT WITH 6" THICK DROP PANELS AND 15" SLAB SHALL HAVE #7@12" EW CONTINUOUS BOTTOM MAT WITH 8" THICK DROP PANELS. 18" SLAB SHALL HAVE #7@10" EW CONTINUOUS BOTTOM MAT. SEE PLAN FOR SLAB THICKNESS. REBAR OUTLAYERS SHALL RUN IN THE NORTH-SOUTH DIRECTION.
- REBAR DESIGNATION WITHOUT "T" STANDS FOR BOTTOM BARS. ADDITIONAL REINFORCING SHOWN IN PLAN AS #6T STANDS FOR 6 NO. 6 BARS AT TOP SPACED WITHIN THE STRIP IT IS SHOWN.
- SEE TYPICAL DETAILS FOR REBARS LAYOUT.
- SEE BEAM SCHEDULES ON S0202 FOR BEAM REINFORCING.
- STANDS FOR 6" LIGHT GAUGE STUD BEARING WALL FRAMING TO BE DESIGNED BY LIGHT GAUGE CONTRACTOR.

STRUCTURAL PLANS CERTIFIED AS PROVIDED IN SECTION 106.1.4.1 OF THE D.C. CONSTRUCTION CODES

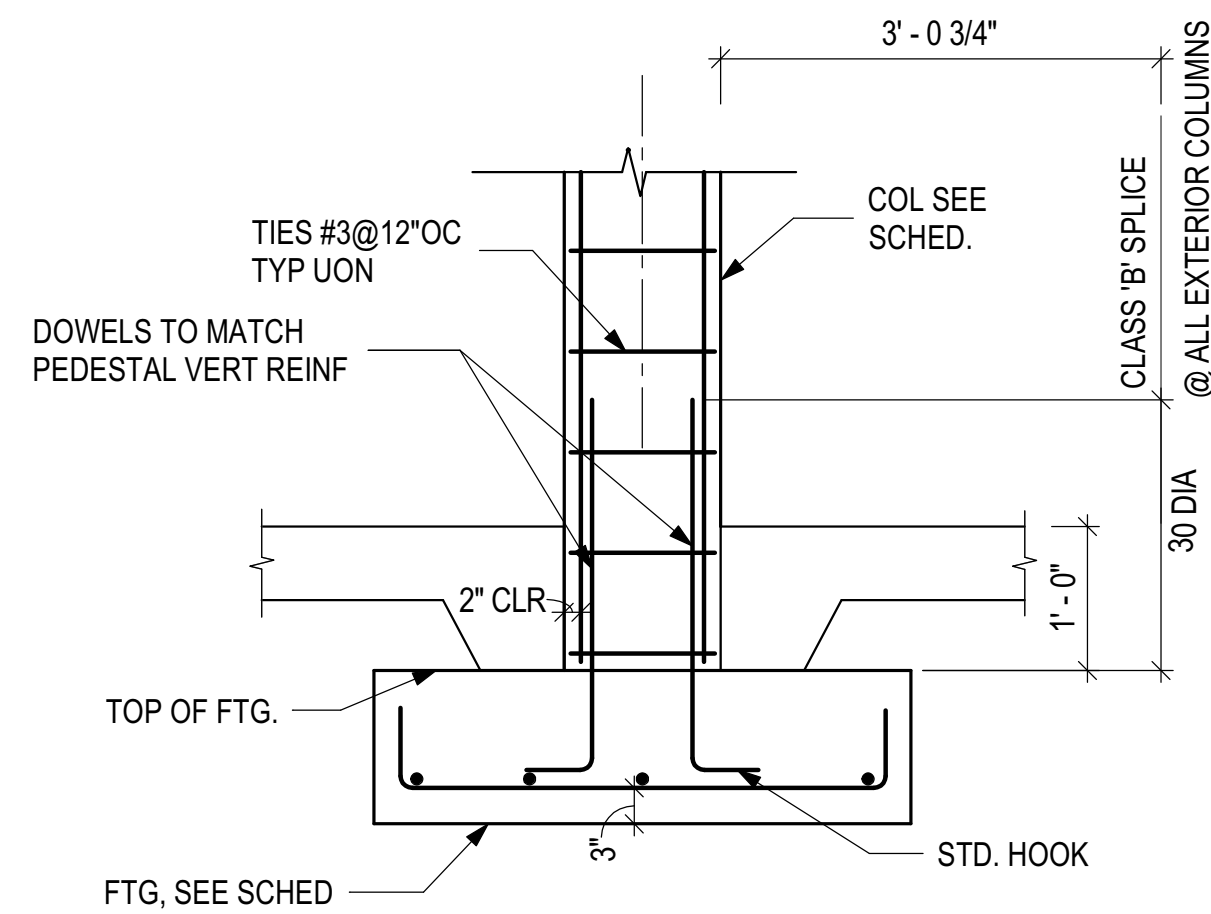
GROUND FLOOR FRAMING PLAN

1/8" = 1'-0"

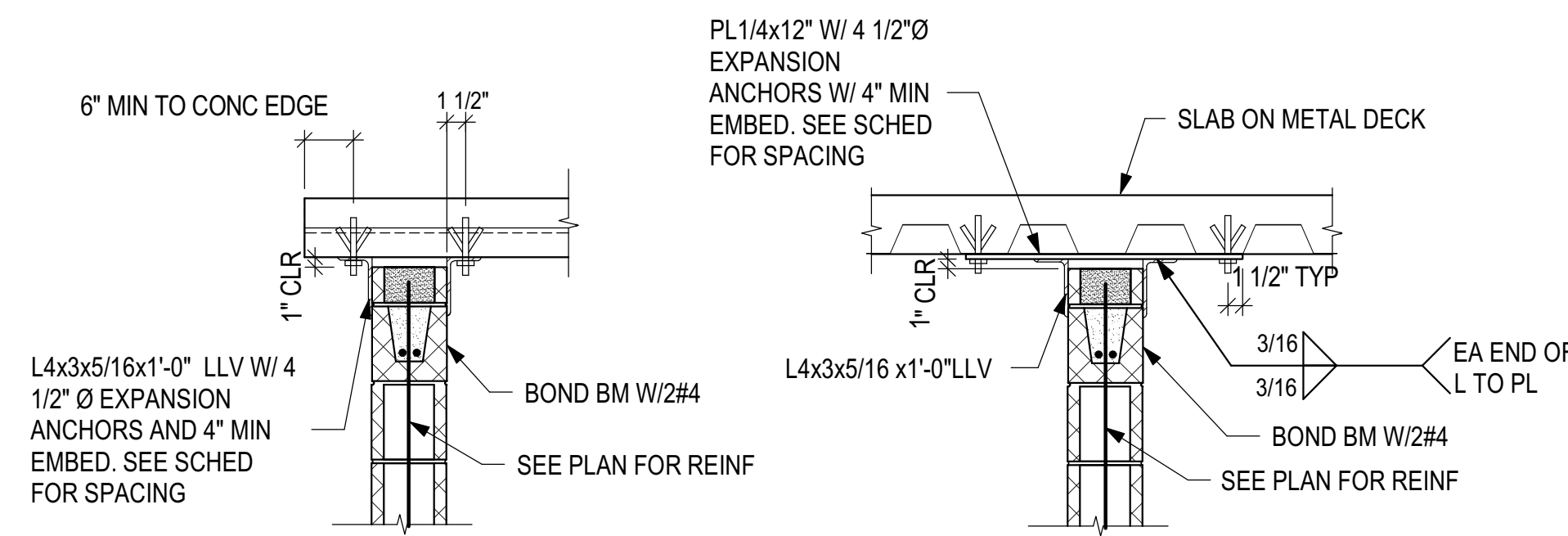


RETAINING WALL SCHEDULE								
STEM HT (H)	As	Ab	C	W	M	O	P	D
H ≤ 4'-0"	8"	12"	1'-0"	2'-8"	#4@9"	#4@9"	#4@9"	#4@9"
4'-0" < H ≤ 6'-0"	10"	12"	1'-8"	3'-7"	#4@9"	#4@9"	#4@9"	#4@9"

TYPICAL DETAIL OF SITE RETAINING WALL



TYPICAL COLUMN ON FOOTING DETAIL



STEEL DECK PERPENDICULAR TO WALL

STEEL DECK PARALLEL TO WALL

TOP OF WALL CONNECTION SPACING SCHEDULE		
MAXIMUM WALL THICKNESS	MAXIMUM WALL HEIGHT	TOP CONNECTION SPACING
8"	18'-0"	4'-0"
12"	13'-0"	4'-0"
12"	17'-0"	3'-0"
12"	22'-0"	2'-0"

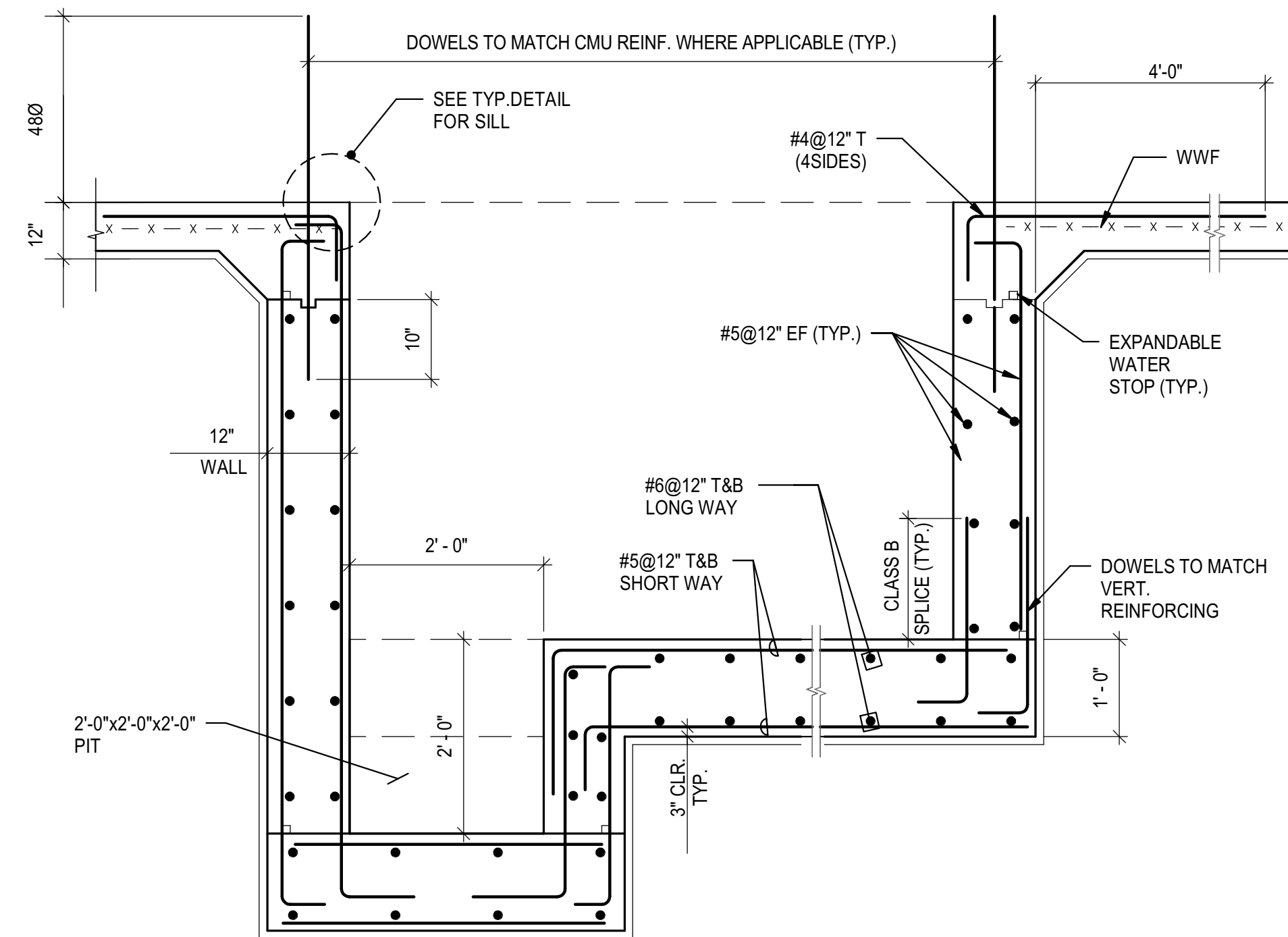
NOTES:

- WHERE FILLER IS REQUIRED PER ARCHITECTURAL REQUIREMENTS, PROVIDE FILLER WITH MINIMUM 50 PERCENT COMPRESSIBILITY AND THE GAP SHALL BE MAINTAINED AFTER FILLER IS COMPRESSED.
- FIREPROOFING REMOVED OR DAMAGED BY INSTALLATION OF TOP OF WALL CONNECTIONS SHALL BE REPLACED.
- TYPICAL EXPANSION ANCHOR SHALL BE HILTI KWIK BOLT TZ OR APPROVED EQUIVALENT.

BAR LAP SPLICE LENGTH SCHEDULE		
GRADE 60 BARS	CAST-IN-PLACE CONCRETE STRENGTH, PSI	
	4000	5000
#3	24"	21"
#4	32"	29"
#5	40"	36"
#6	48"	43"
#7	70"	62"
#8	80"	71"
#9	90"	80"
#10	101"	90"
#11	112"	100"

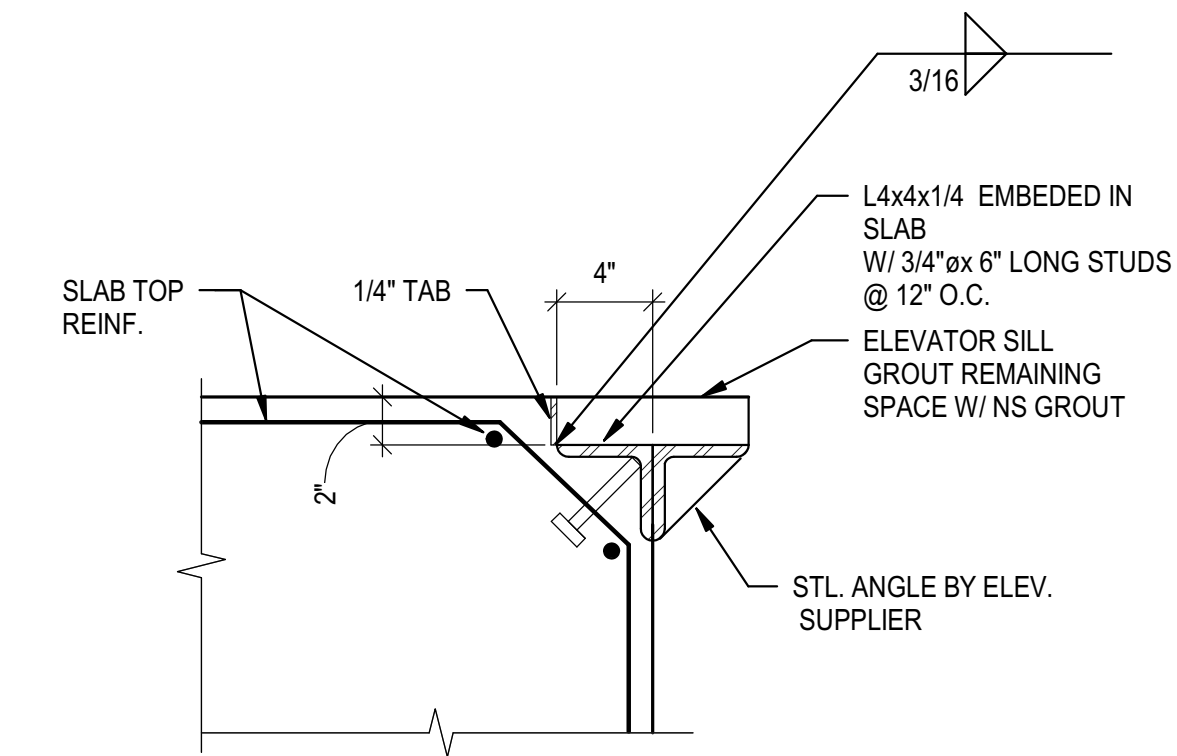
NOTE: LENGTHS MAY BE MULTIPLIED BY 0.77 FOR BARS OTHER THAN TOP SLAB OR BEAM BARS. LENGTHS MAY ALSO BE MULTIPLIED BY 0.80 WHEN SPECIFIED AS EMBEDMENT. WHERE BAR TO BAR SPACING < 3db MULTIPLY LENGTHS BY 1.5. SPLICED GRADE WALL STIRRUPS ARE TO BE LAP SPLICED WITH NO REDUCTION. ALL EPOXY COATED BAR LAPS TO BE INCREASED BY 1.3 IN ADDITION TO ALL OTHER LENGTH ADJUSTMENTS.

TYPICAL CMU WALL TOP CONNECTION DETAILS



TYP. ELEVATOR PIT DETAIL

LAP SPLICE OR EMBEDMENT LENGTH FOR REINFORCING BARS



REFER TO ARCH. DWGS. FOR LOCATIONS, LENGTH & DETAILS.

TYP. ELEVATOR SILL DETAIL

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Issues / Revisions

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Drawing Title

TYPICAL DETAILS

Scale	As indicated	Drawn By	CS
Date	12/07/2018	Checked By	

S0302

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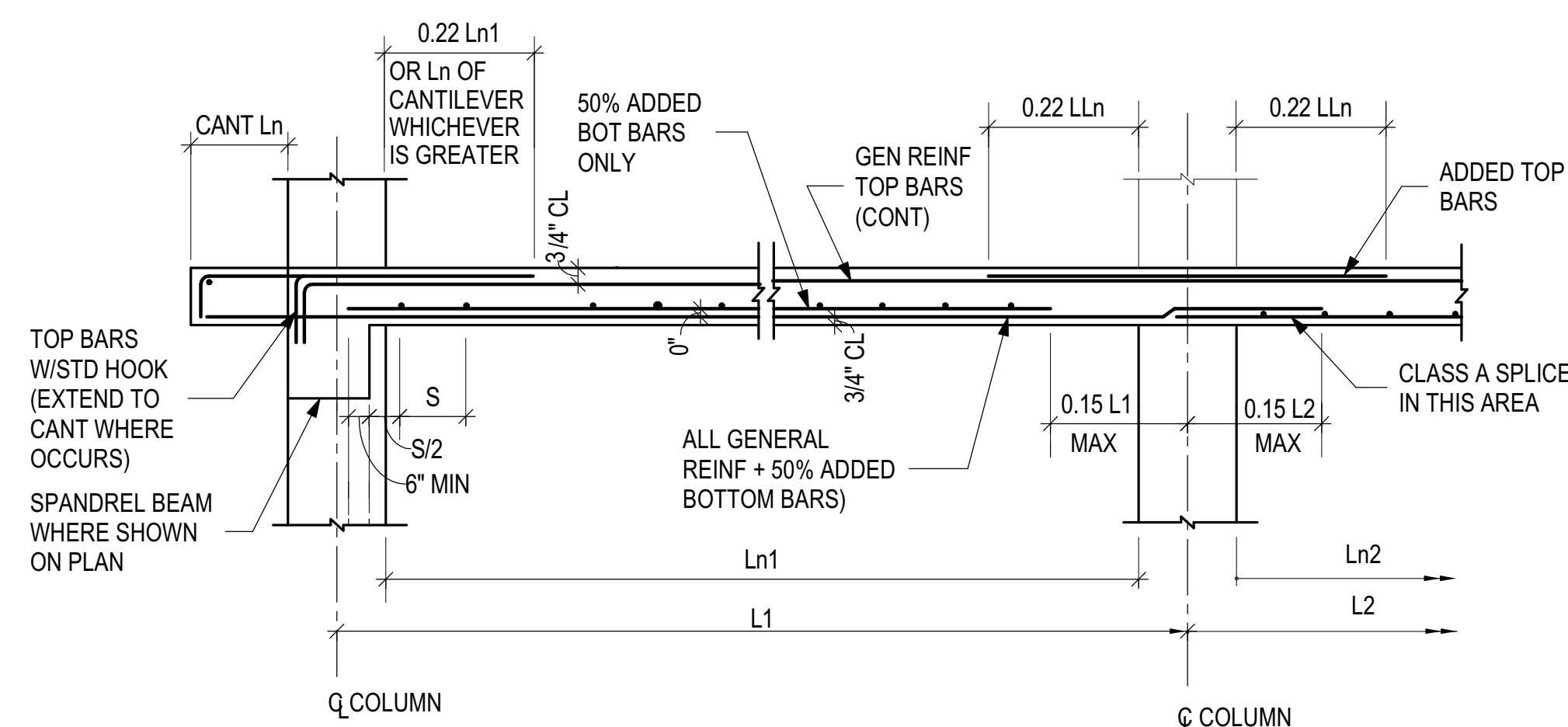
CQA#2018038

Drawing Title

TYPICAL DETAILS

Scale: As indicated
 Date: 12/07/2018
 Drawn By: CS
 Checked By: BY

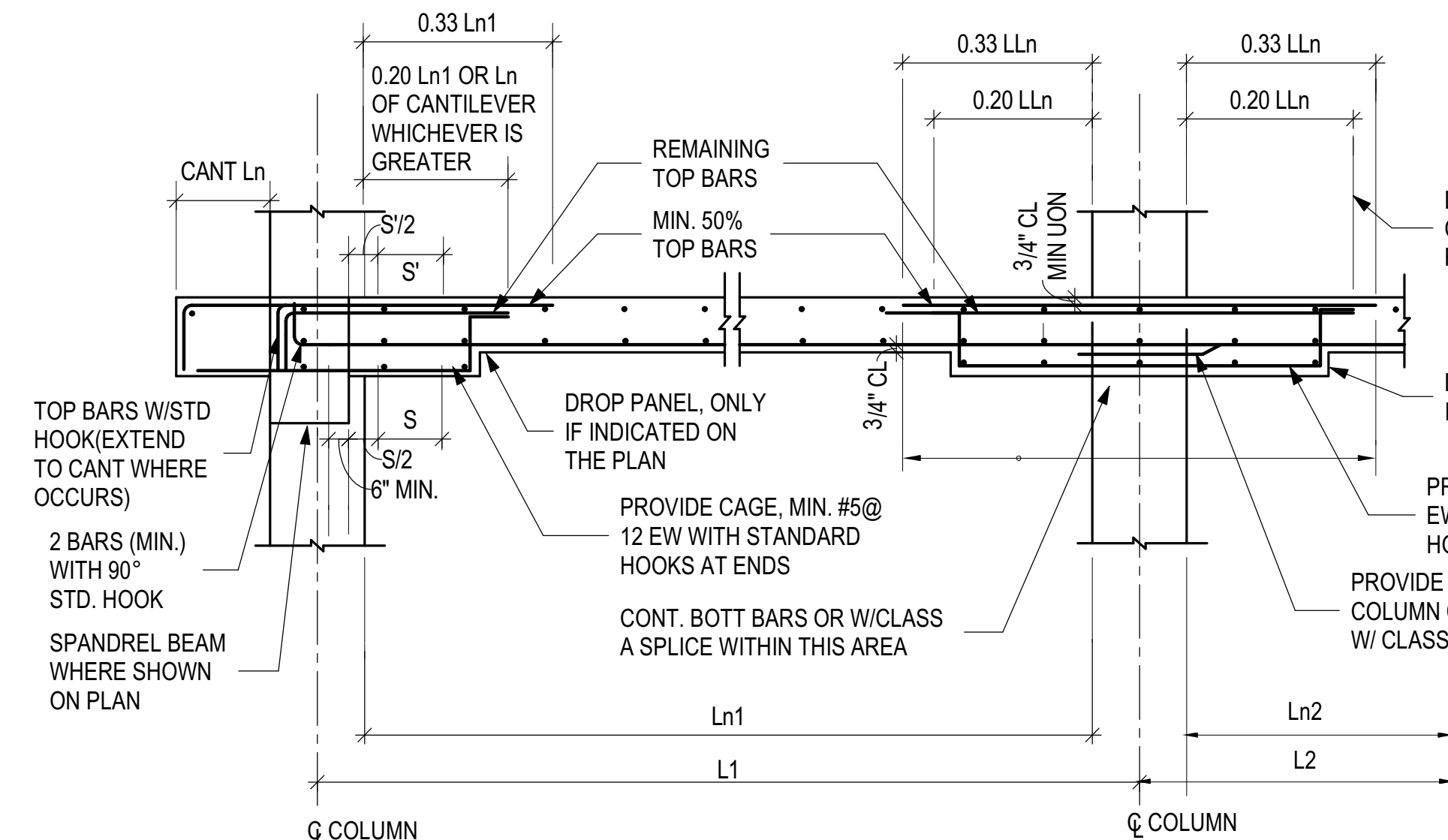
S0303



NOTES:

1. L_{Ln} DENOTES LONGER OF L_{n1} AND L_{n2}.
2. THE GENERAL OR CONTINUOUS REINFORCING (TOP & BOTTOM) CALLED ON PLANS SHALL BE CONTINUOUS UTILIZING THE LONGEST REBARS PRACTICAL AND SHALL BE SPLICED ONLY AS NECESSARY AS FOLLOWS: ALL SPLICES SHALL BE CLASS A SPLICES. ALL TOP REINFORCING SHALL BE SPLICED AT MID POINT BETWEEN COLUMNS. ALL BOTTOM REINFORCING SHALL BE SPLICED AT COLUMN LOCATIONS.

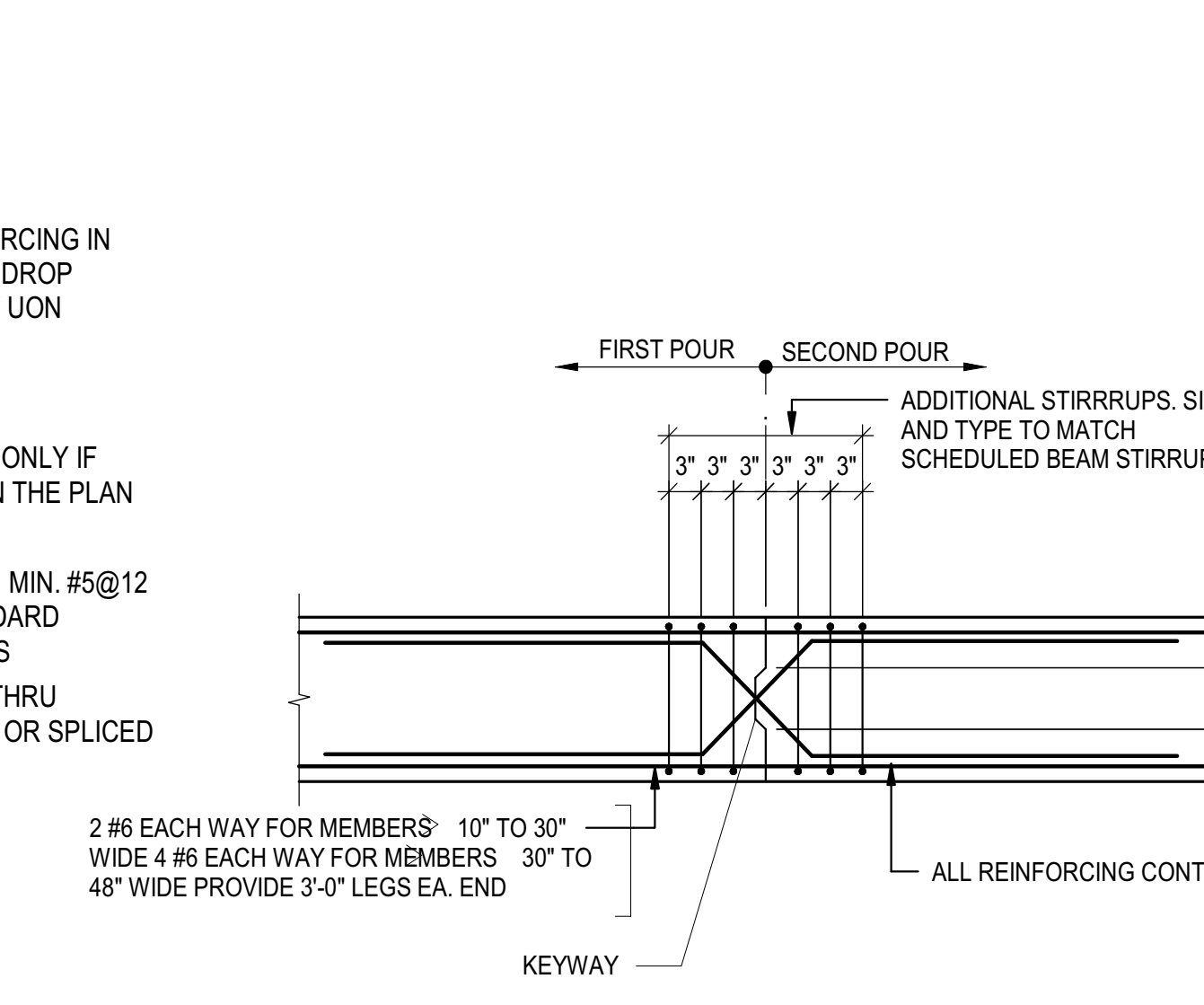
TYPICAL MIDDLE STRIP REINFORCING DETAIL



NOTES:

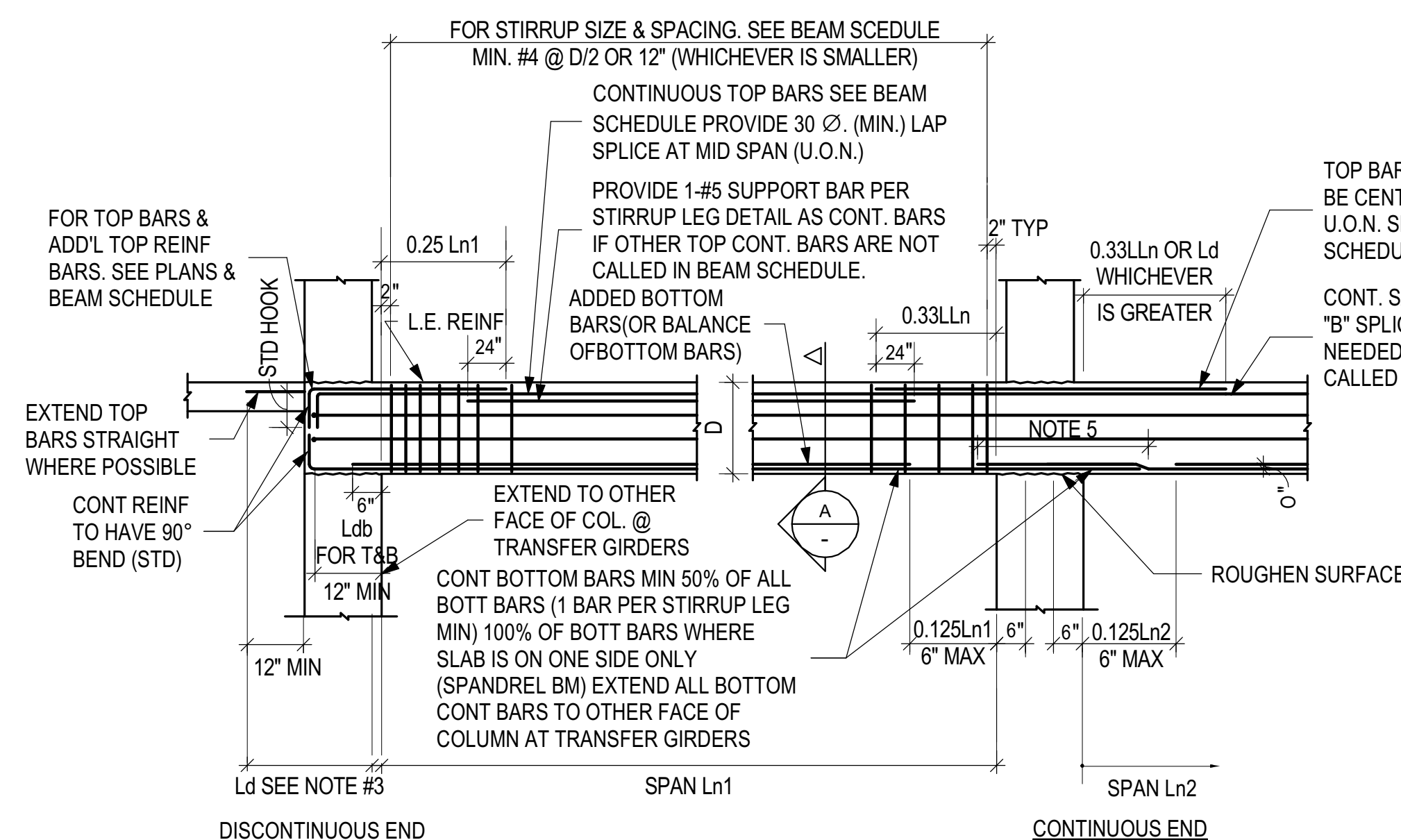
1. L_{Ln} DENOTES LONGER OF ADJACENT CLEAR SPAN L_{n1} AND L_{n2}.
2. THE GENERAL OR CONTINUOUS REINFORCING CALLED ON PLAN SHALL BE CONTINUOUS UTILIZING THE LONGEST REBARS PRACTICAL & SHALL BE SPLICED ONLY AS NECESSARY AS SHOWN IN THE DETAIL.
3. DROP PANEL SIZES INDICATED ON PLANS ARE MINIMUM REQUIRED, & CAN BE ADJUSTED AS FOLLOWS TO FACILITATE FORMWORK: PLAN DIMENSIONS +6", DEPTH DIMENSIONS +1".

TYPICAL COLUMN STRIP WITH DROP PANEL REINFORCING DETAIL



NOTE: JOINT SHALL BE WITHIN MIDDLE THIRD OF SPAN.

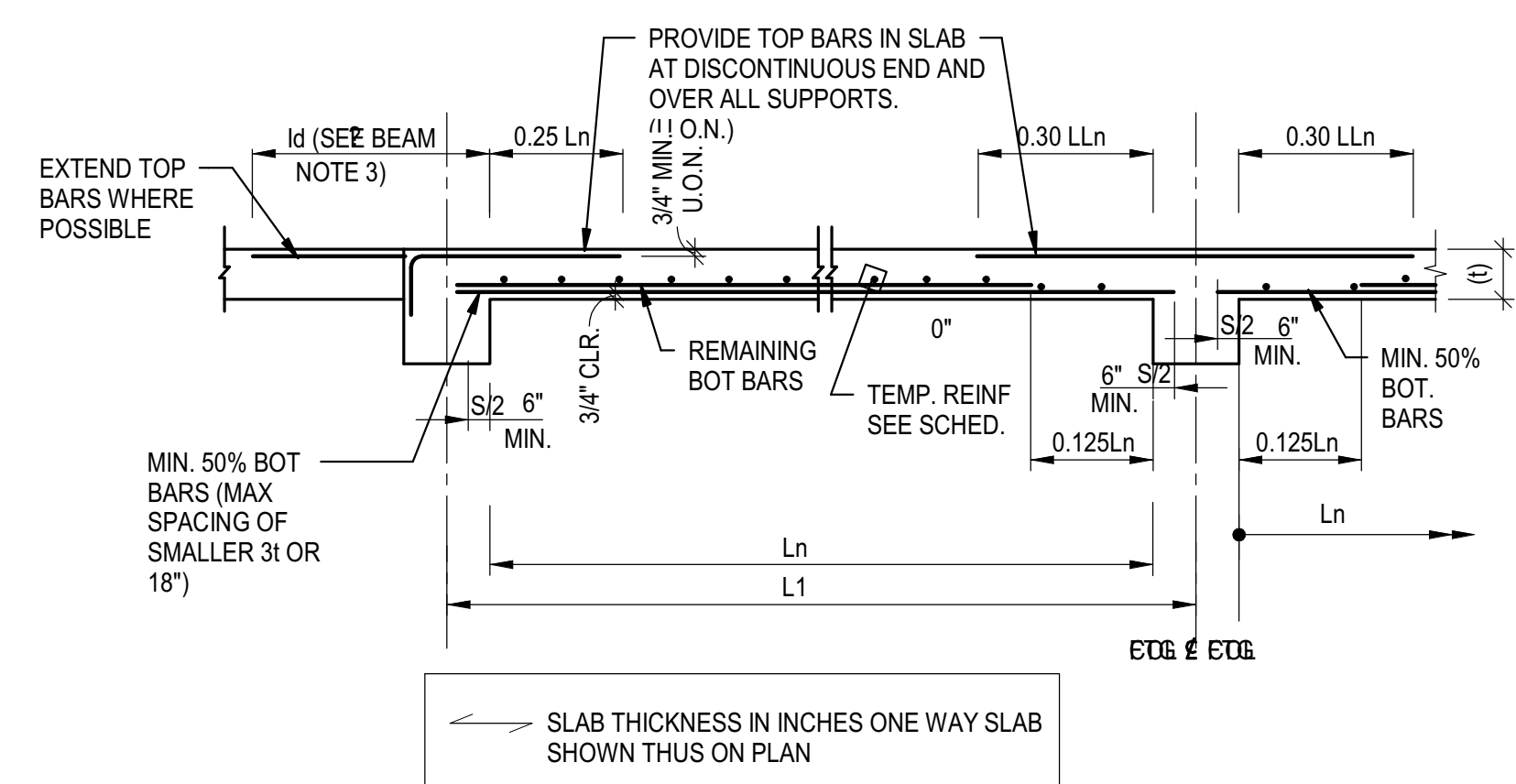
TYPICAL CONST JOINT AT CONC. BEAM REINF DETAIL



NOTES:

1. CARRY CONT. BEAM REINF W/O SPLICE AS LONG PRACTICAL. SPLICES FOR CONT BEAM REINF TO BE MADE AT MID-SPAN FOR TOP REINF AT SUPPORT FOR BOTTOM REINF (U.O.N.).
2. IN ALL SPANDREL BEAMS A MIN OF 2-BARS IN TOP & 2-BARS IN BOT SHALL BE CONTINUOUS.
3. L_d DENOTES TENSION DEVELOPMENT LENGTH, AND L_{Ln} DENOTES LONGER OF L_{n1} AND L_{n2}.
4. ANY BEAM NOT SCHEDULED SHALL HAVE 1% BOT REINF & 2-#6(T) CONT.
5. PROVIDE CLASS "B" SPLICE (OR CONTINUOUS BARS) FOR 2 BOTTOM BARS MIN.

TYPICAL CONCRETE BEAM REINFORCING DETAIL

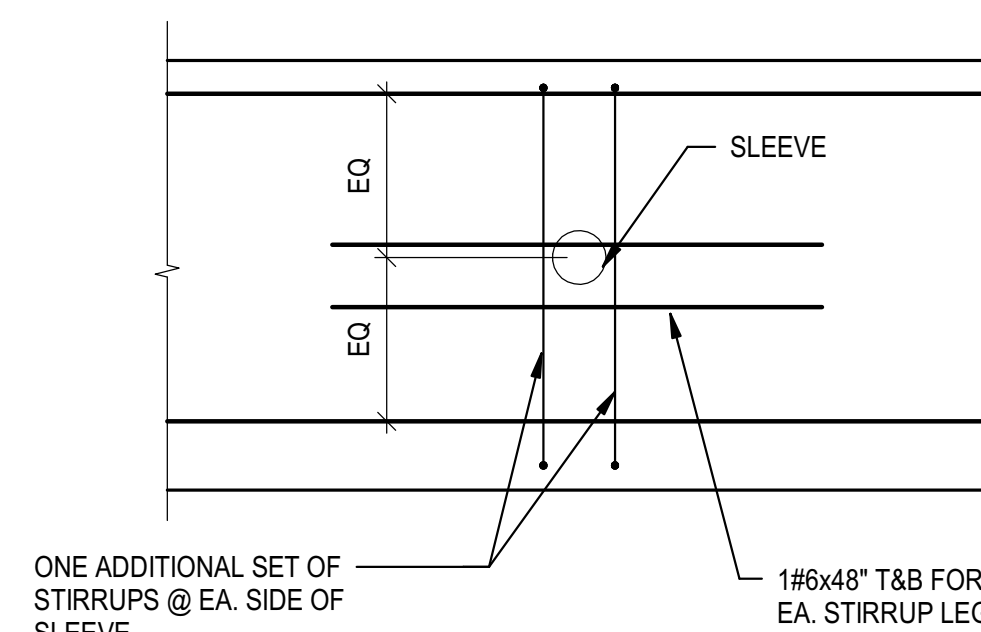


- NOTE: 1. L_{Ln} DENOTES LONGER OF ADJACENT CLEAR SPAN
 2. TEMP BARS TO BE SPLICED W/ CLASS A SPLICE

TYPICAL ONE-WAY SLAB DETAIL

ONE WAY SLAB TEMP REINF SCHEDULE	
6"	#3 @ 10"
6 1/2"	#3 @ 9"
7"	#3 @ 8"
7 1/2"	#5 @ 14"
8"	#5 @ 13"
8 1/2"	#5 @ 13"
9"	#5 @ 12"
9 1/2"	#5 @ 12"
10"	#5 @ 11"
10 1/2"	#5 @ 10"
11"	#5 @ 10"
11 1/2"	#5 @ 9"
12"	#5 @ 9"

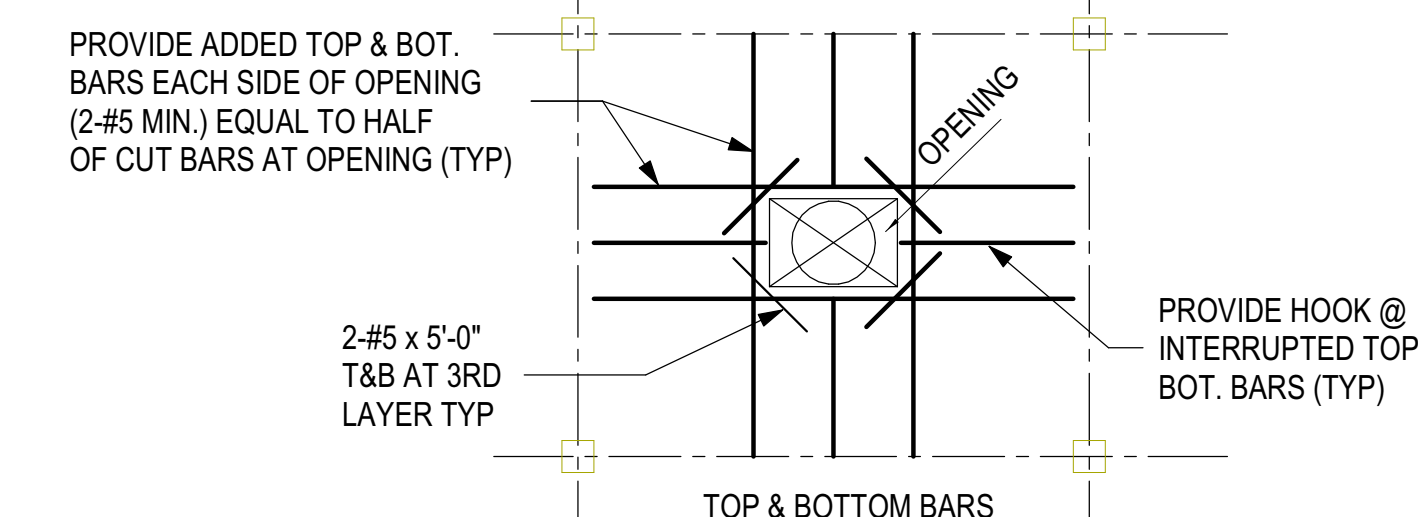
TYPICAL CONCRETE BEAM SLEEVE REINFORCING DETAIL



NOTES:

1. SLEEVES ARE PERMITTED ONLY IN THE MIDDLE THIRD OF THE CLEAR SPAN
2. ALL SLEEVES MUST BE CLEARLY INDICATED ON THE REINFORCING SHOP DWGS.
3. NO SLEEVES WILL BE PERMITTED WITHIN 24" FROM A TRANSFERRED COLUMN.
4. SLEEVE DIAMETER CANNOT EXCEED 10% OF BEAM DEPTH U.O.N.

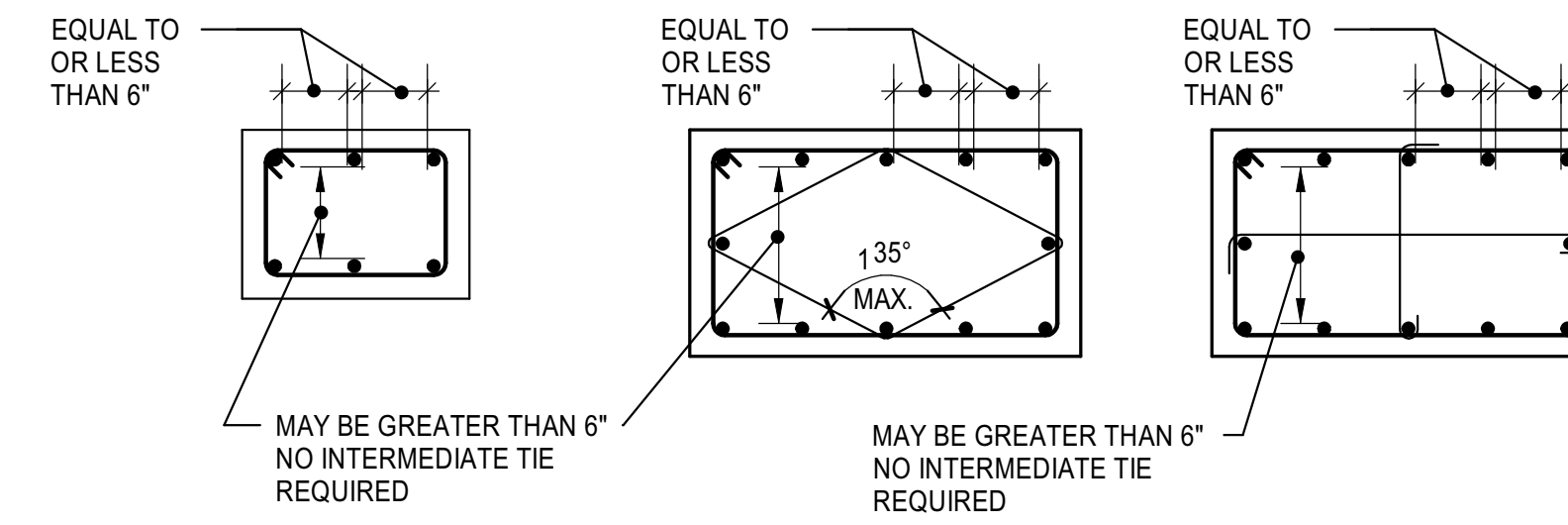
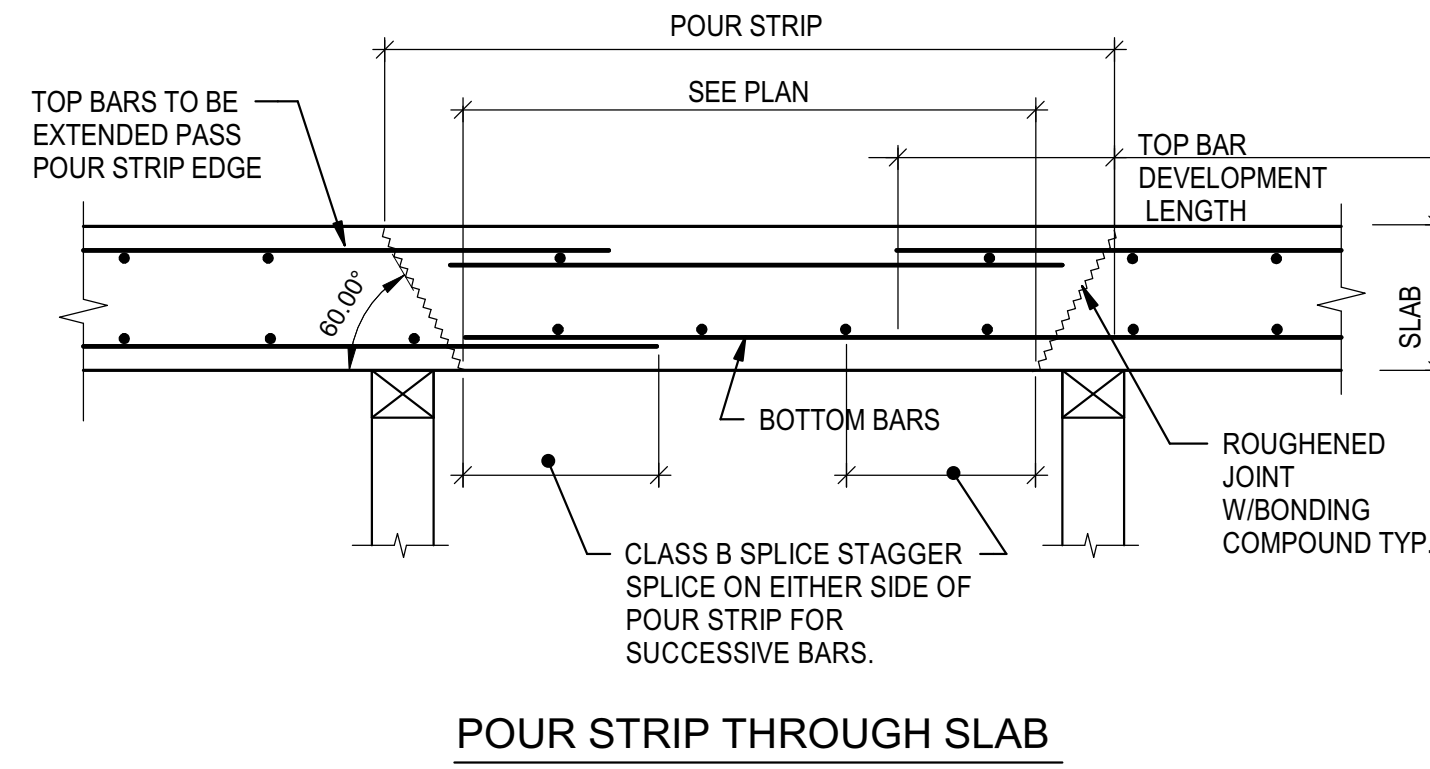
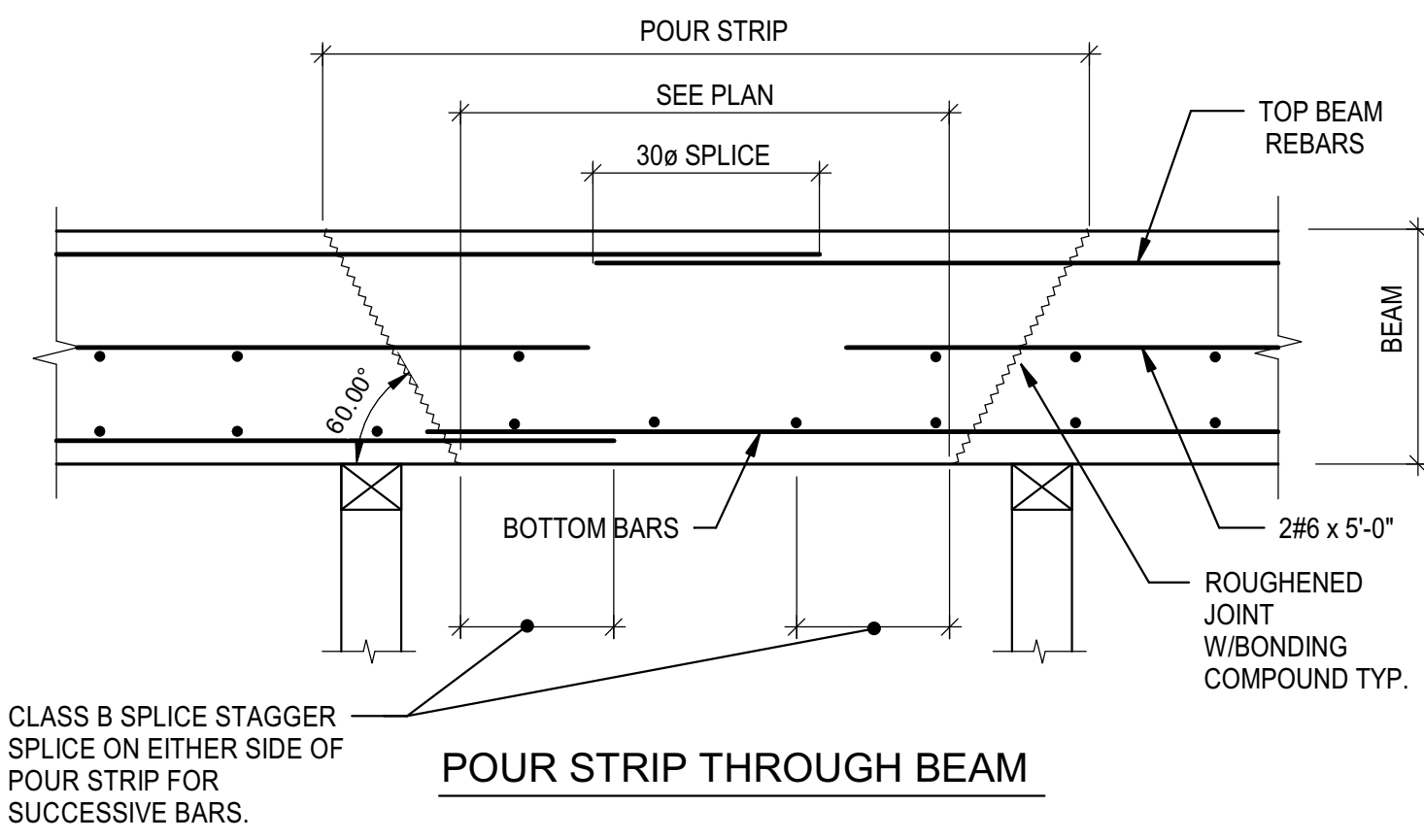
TYPICAL TWO-WAY FLAT SLAB REINFORCING DETAIL



NOTE:

- IF ADDITIONAL OPENINGS OTHER THAN THOSE CALLED ON PLAN ARE REQUIRED, THE CONTRACTOR SHALL SUBMIT A LAYOUT OF THOSE OPENINGS FOR APPROVAL PRIOR TO OR AT SAME TIME OF SUBMITTING REINF STEEL SHOP DWGS. IF TOP BARS DO NOT OCCUR, PROVIDE #5 @ 12" INTO SLAB WITHIN 4 FEET AT ALL SIDES OF OPENINGS

TYPICAL OPENING REINFORCING DETAIL

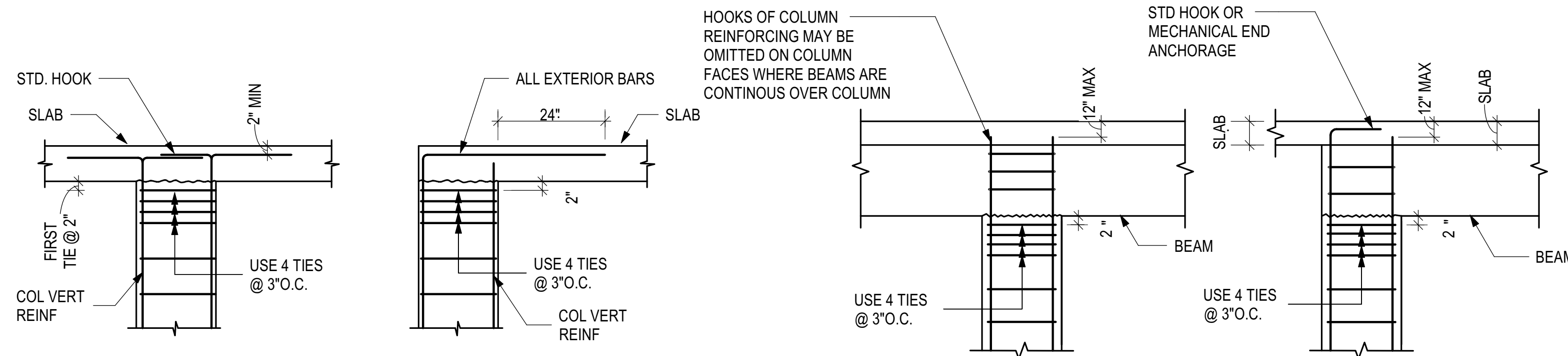


COLUMN TIE SIZE AND SPACING		
LONGITUDINAL BAR SIZE	TIE BAR SIZE	TIE SPACING *
#7	#3	14"
#8	#3	16"
#9	#3	18"
#10	#3	18"
#11 OR LARGER	#4	22"

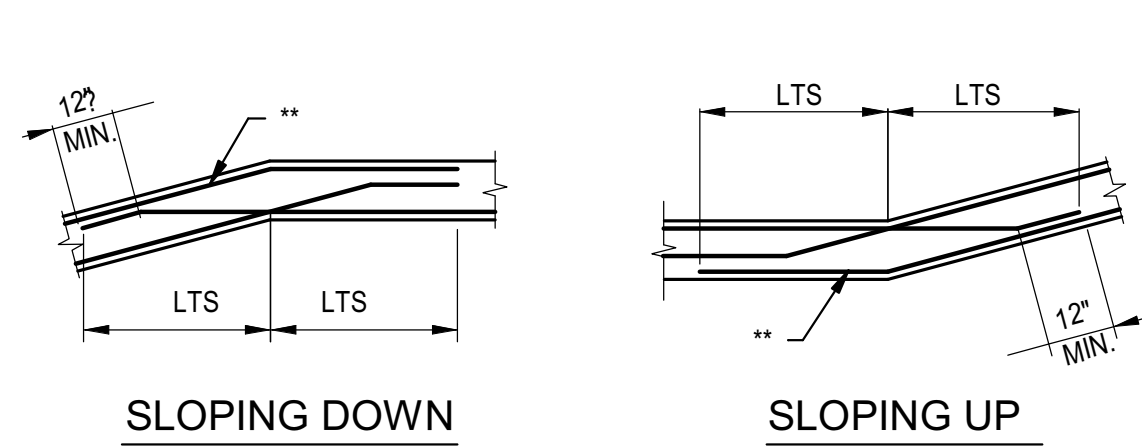
* TIE SPACING SHALL NOT EXCEED THE LEAST DIMENSION OF COLUMN/PEDESTAL

- NOTES:
- AREAS ON EITHER SIDE OF THE POUR STRIP SHALL BE CAST FIRST, AND REMAIN SHORED UNDISTURBED UNTIL POUR STRIP IS CAST & CURED TO 75% OF DESIGN STRENGTH.
 - POUR STRIP AREA SHALL BE CAST NOT EARLIER THAN 30 DAYS AFTER THE LATEST OF EITHER SIDE AREAS IS CAST.
 - ALL BARS CROSSING THE POUR STRIP SHALL BE INTERRUPTED & SPLICED AS REQUIRED WITHIN THE POUR STRIP.
 - PROVIDE CONSTRUCTION JOINTS AT EACH SIDE IN CONCRETE WALLS CROSSING THE POUR STRIP. THE PORTION OF WALL BETWEEN THESE CONSTRUCTION JOINTS SHALL BE CAST WITH OR AFTER THE POUR STRIP IS CAST.

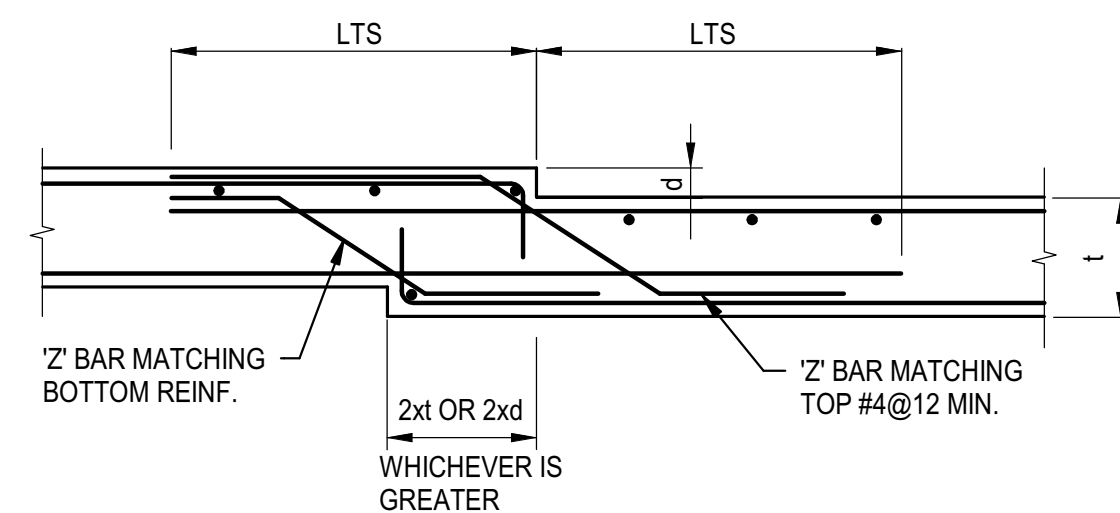
TYPICAL POUR STRIP DETAIL



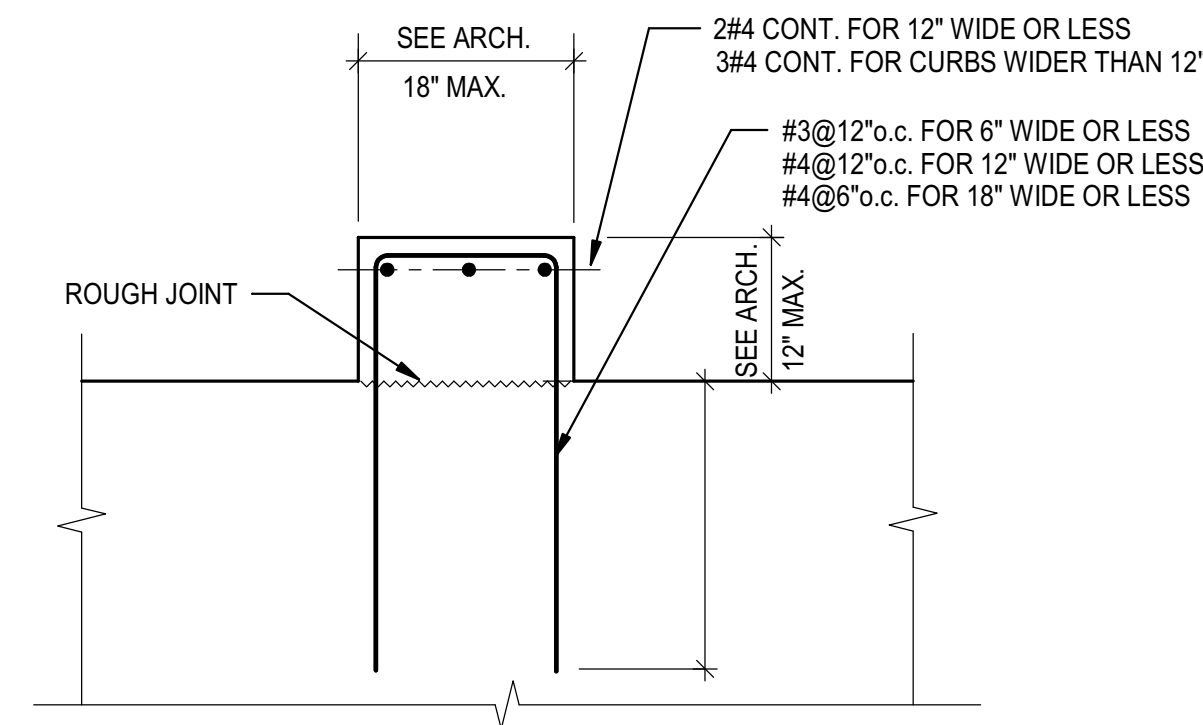
TYPICAL CONCRETE COLUMN REINF DETAIL



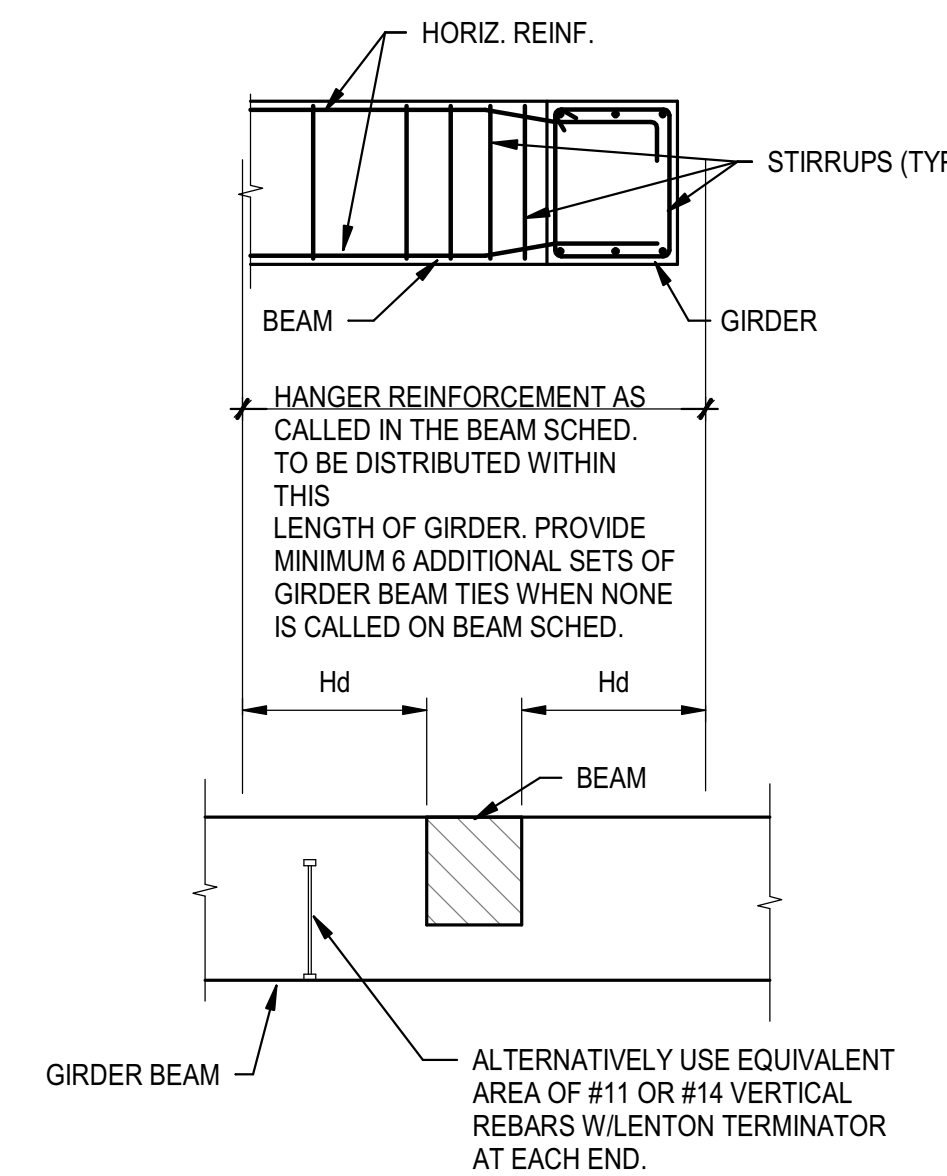
** ANY SPLICE NEEDED FOR THESE BARS TO BE CLASS 'B' LAP SPLICE.



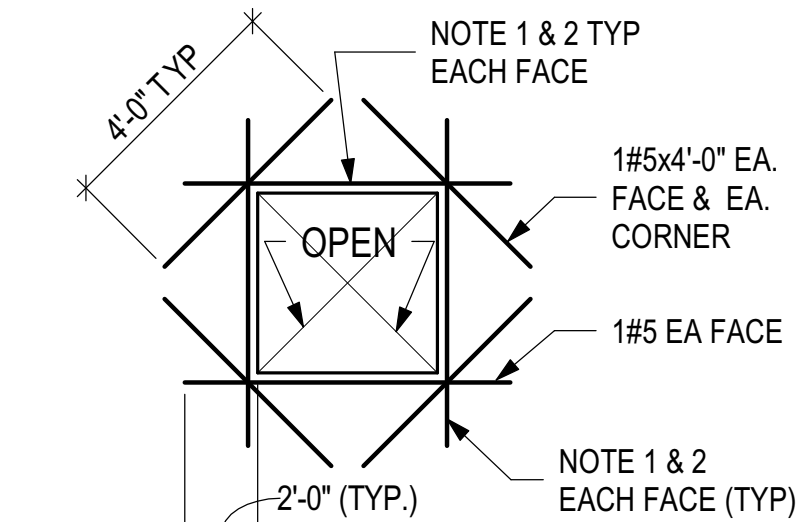
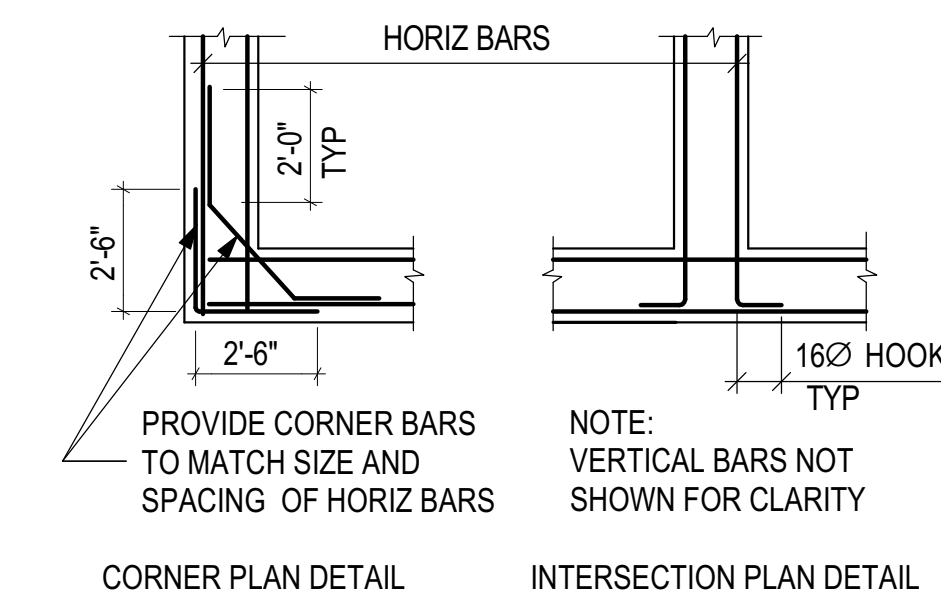
TYPICAL SLAB SLOPE & STEP TRANSITION DETAILS



TYPICAL CURB REINFORCING DETAIL



TYPICAL CONCRETE BEAM TO GIRDER CONN DETAIL



- NOTES:
- PROVIDE ADDL. BARS EQUAL IN AREA TO REINF. CUT @ OPNGS. PLACE HALF OF REINFORCING EA. SIDE + ONE (1) ADDL. (MIN. 2#5 EA. SIDE).
 - ADDL. BARS TO BE SAME LENGTH AS INTERRUPTED BARS WERE ORIGINALLY.

TYPICAL CONCRETE WALL REINF DETAIL

GOVERNMENT OF THE DISTRICT OF COLUMBIA
PERMIT OPERATIONS DIVISION
PLANS APPROVED
Permit No. FD1900028 Date 05/21/19

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CIVIL ENGINEER: Wiles Mensch Corporation 510 8th Street, SE Washington, DC 20003 Phone: 202-638-4040 x255

LANDSCAPE ARCHITECT: Landscape Architecture Bureau 714 7th Street, SE Washington, DC 20003 Phone: 202-543-6550



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Drawing Title

TYPICAL DETAILS

Scale: As indicated
Date: 12/07/2018
Drawn By: CS
Checked By:

S0304

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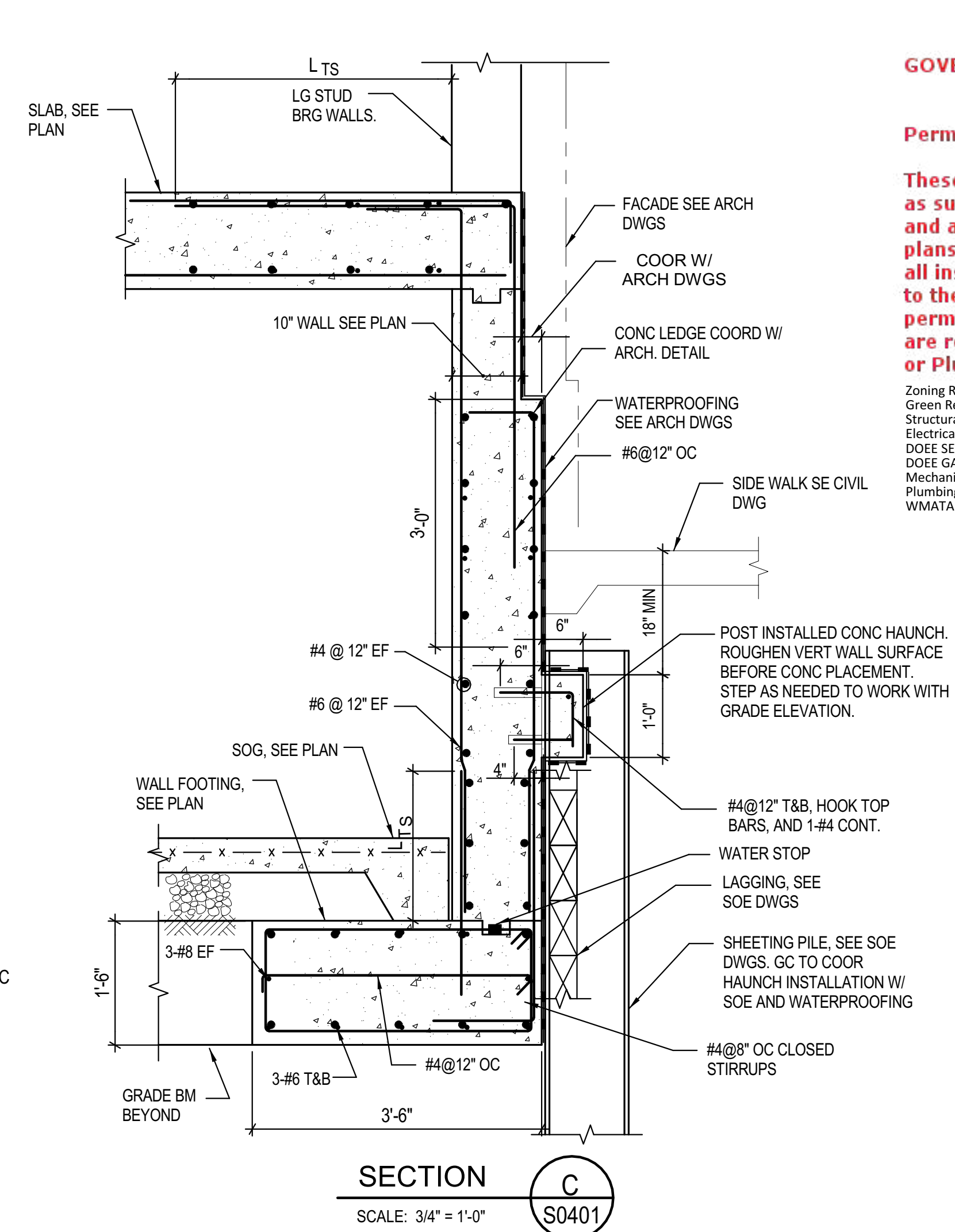
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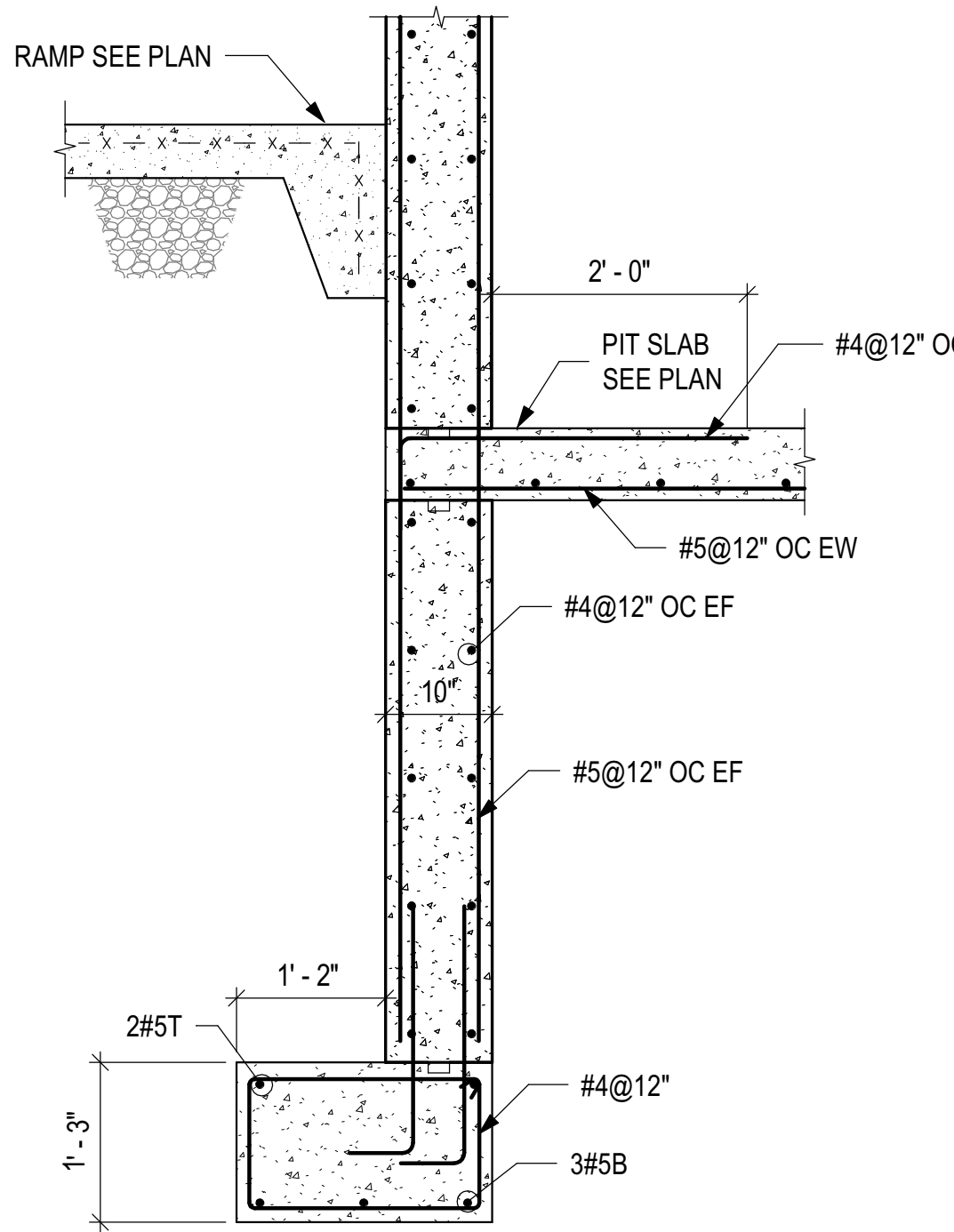
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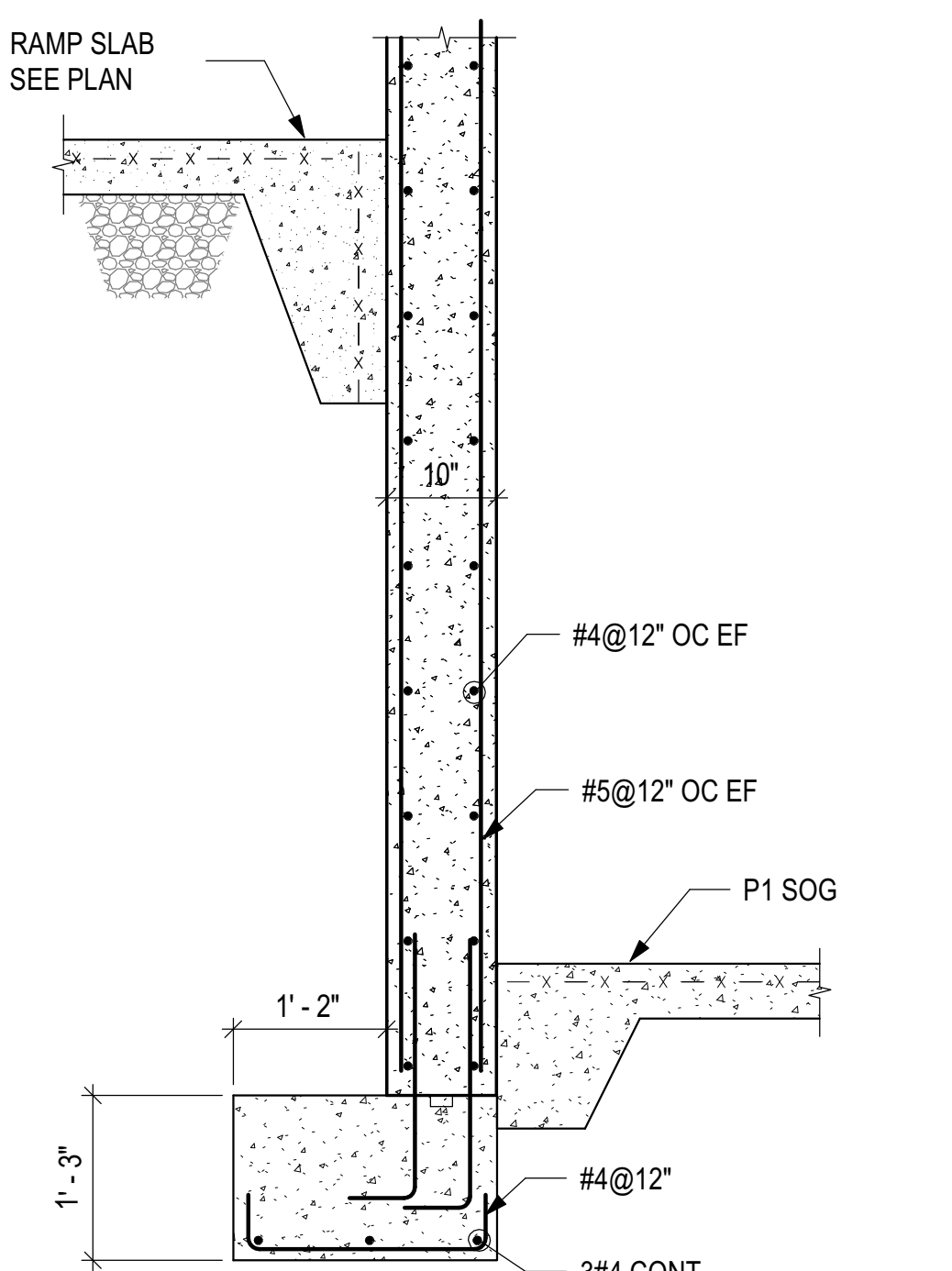
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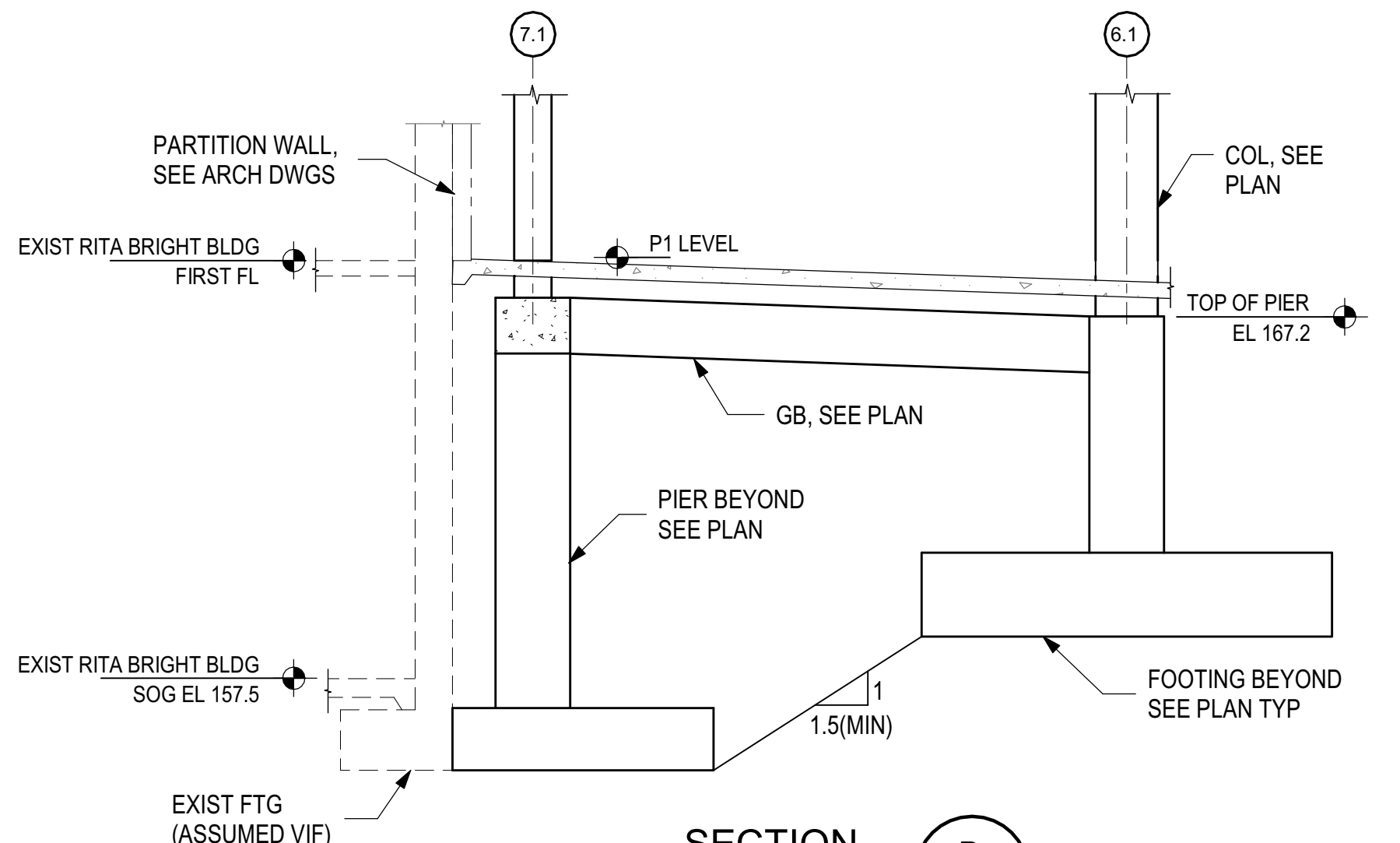
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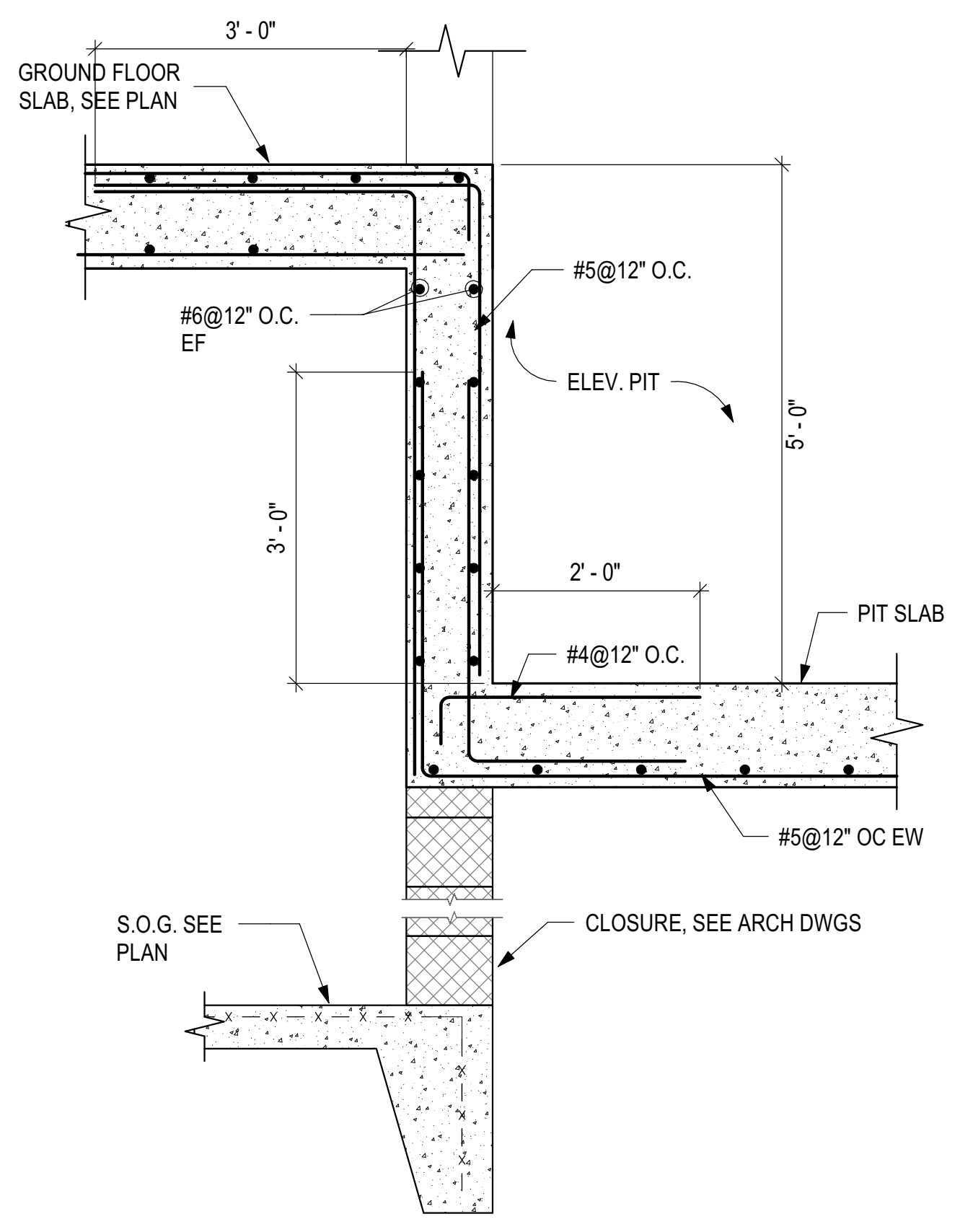
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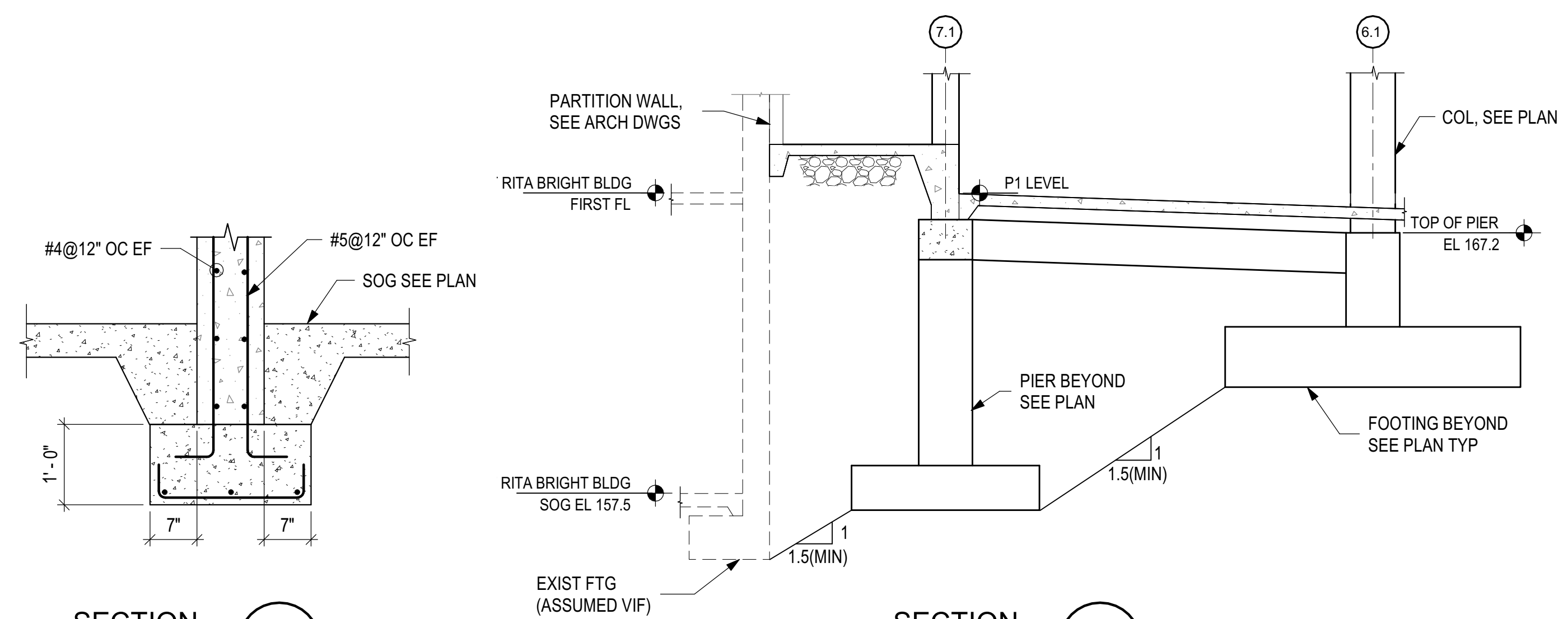
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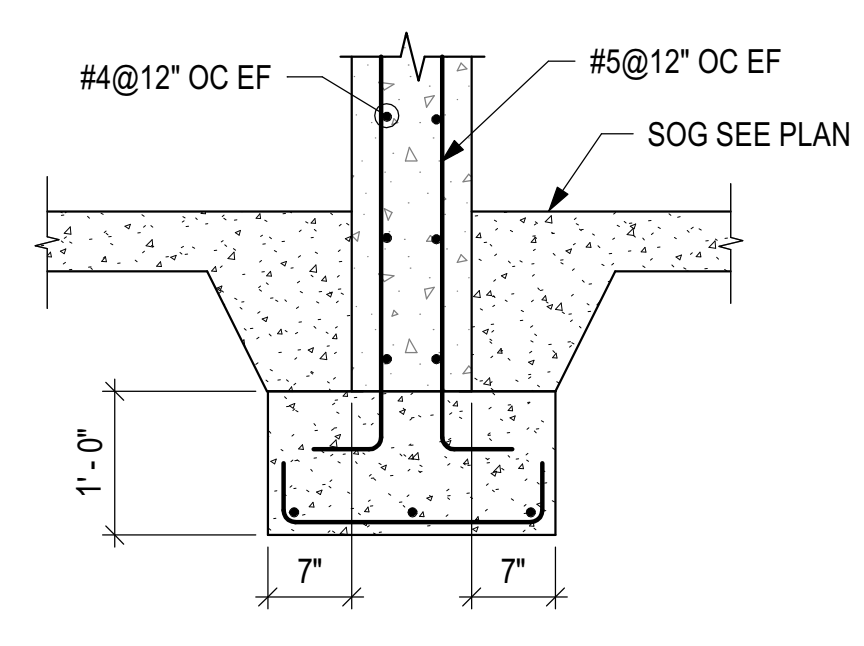
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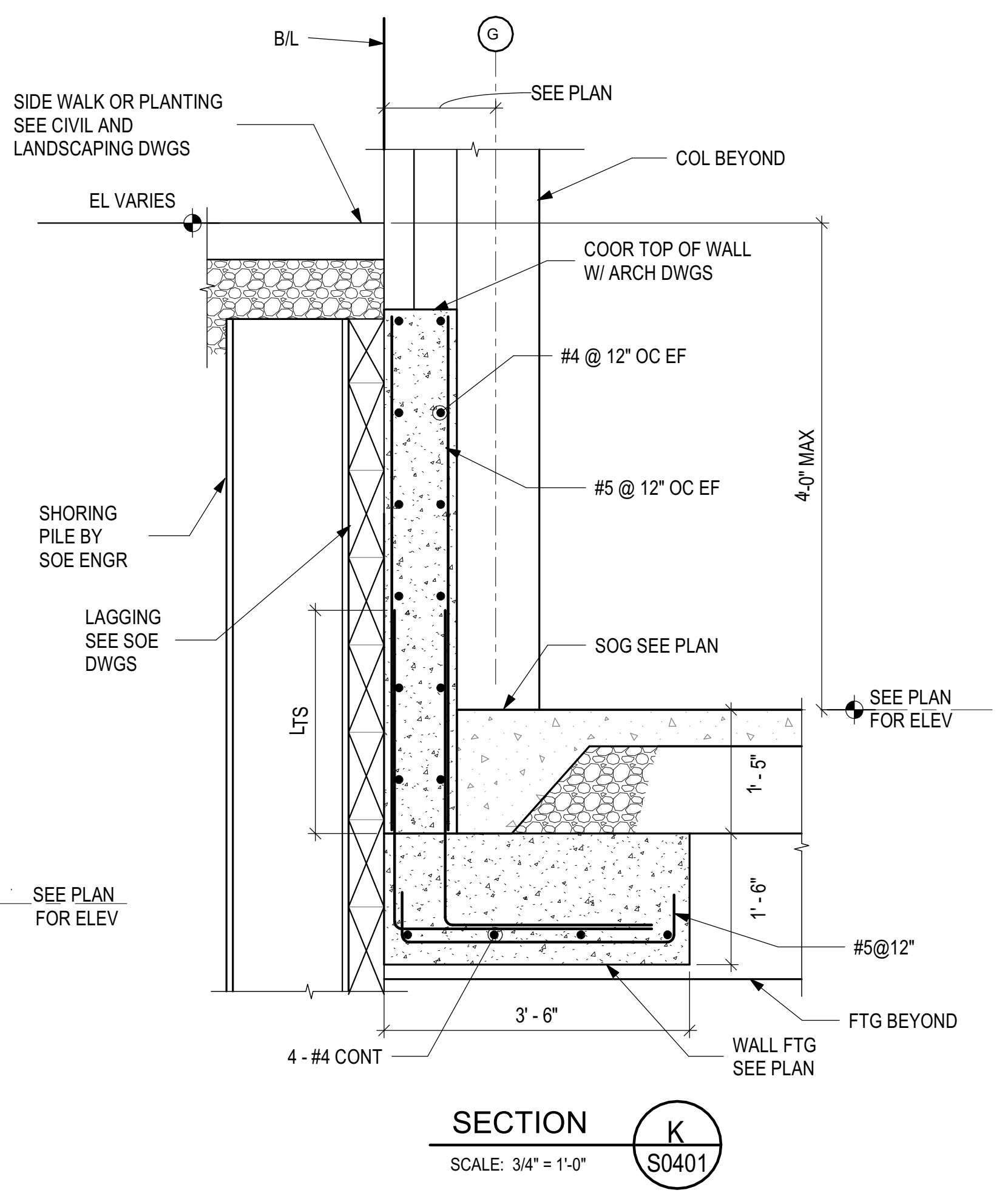
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SECTION H
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 S0401



SECTION G
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 S0401



SECTION K
 SCALE: 3/4" = 1'-0"
 S0401

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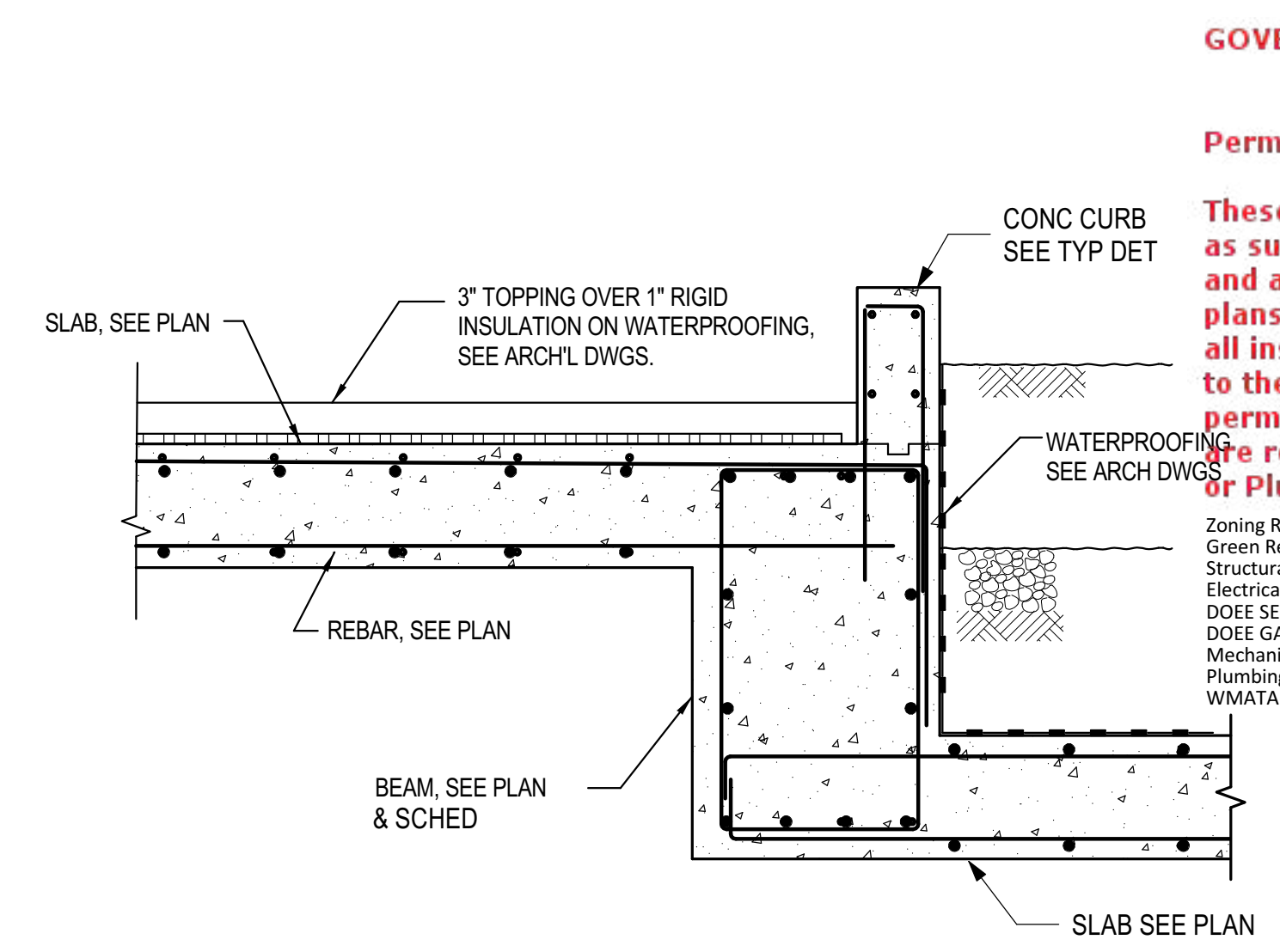
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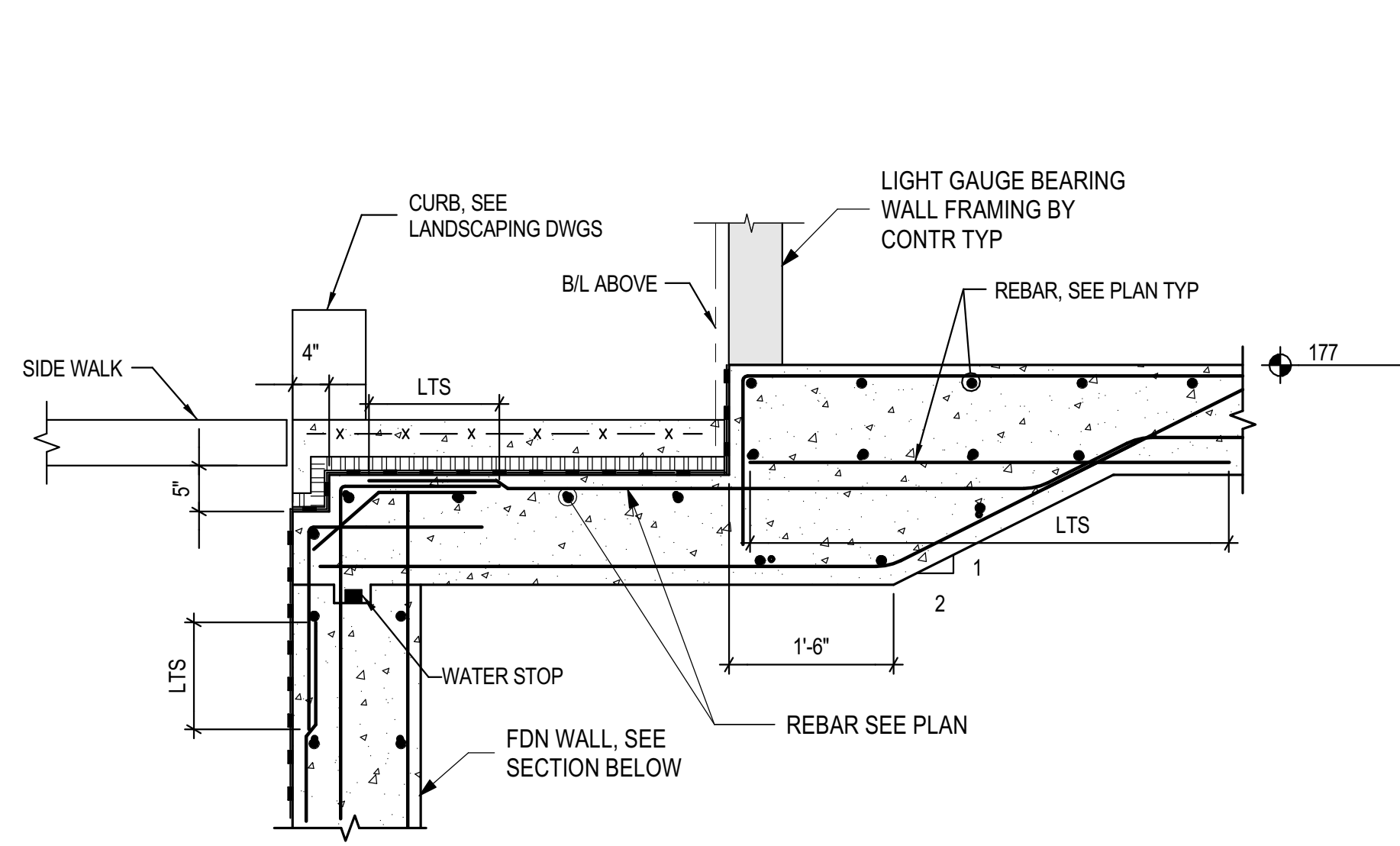
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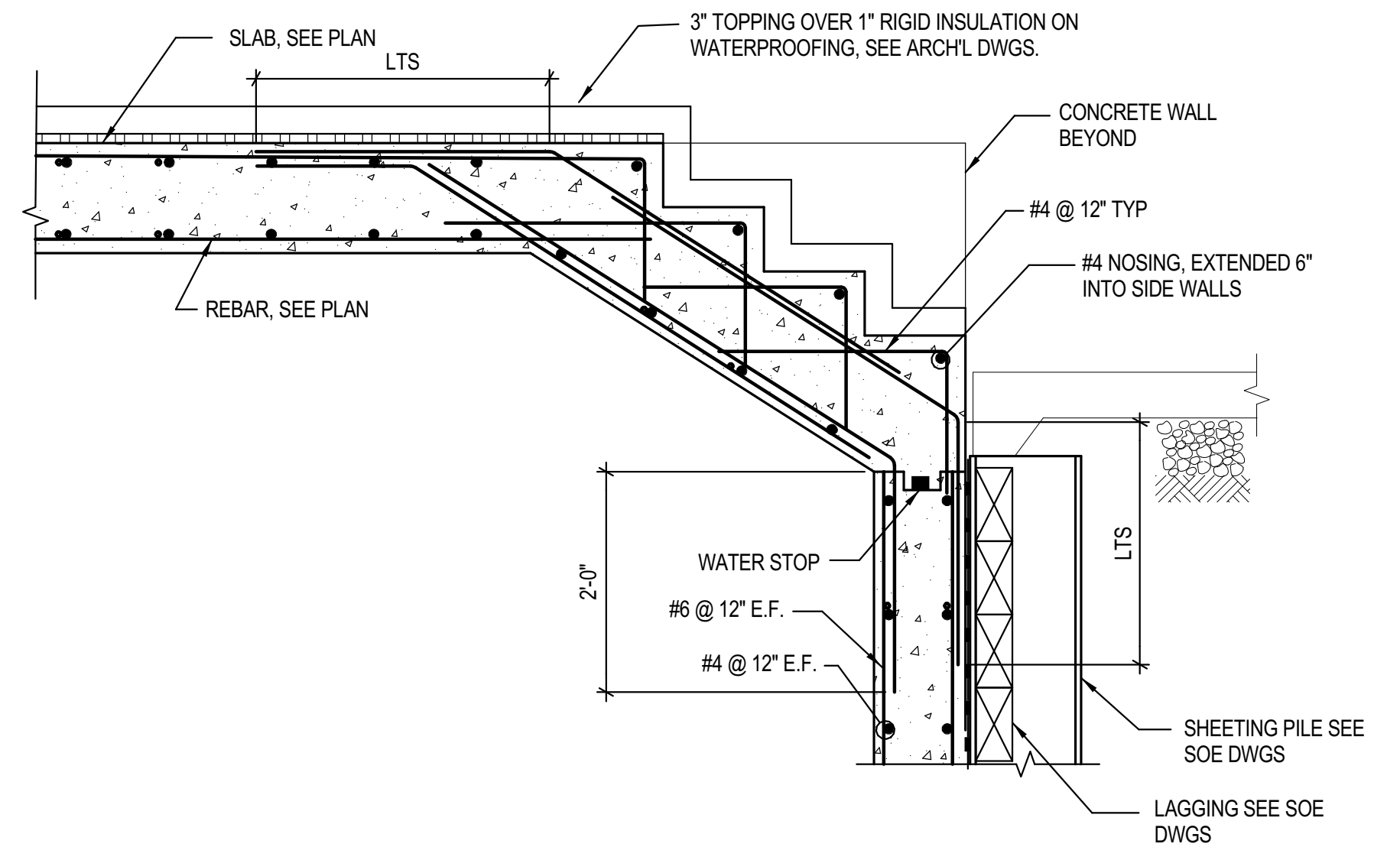
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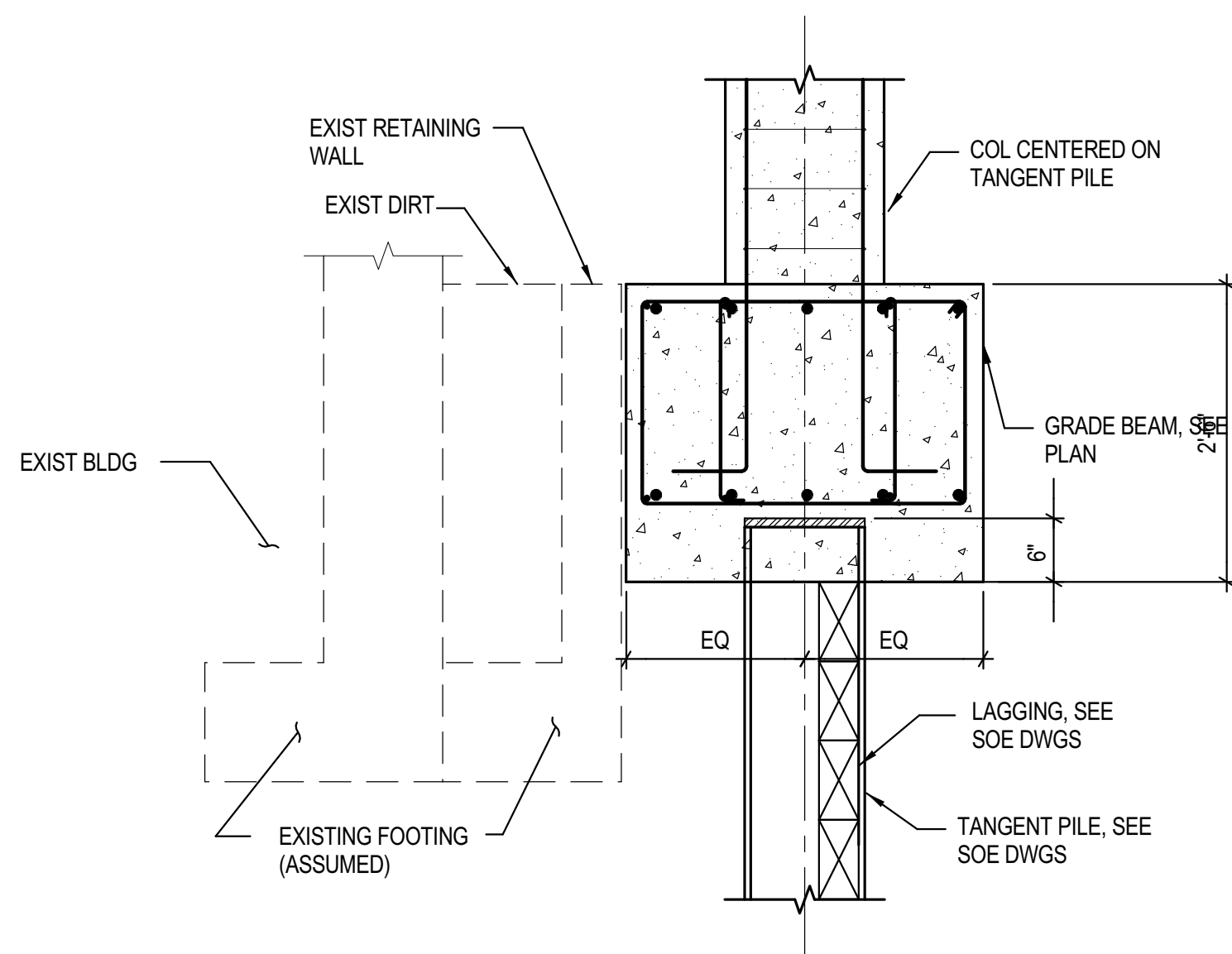
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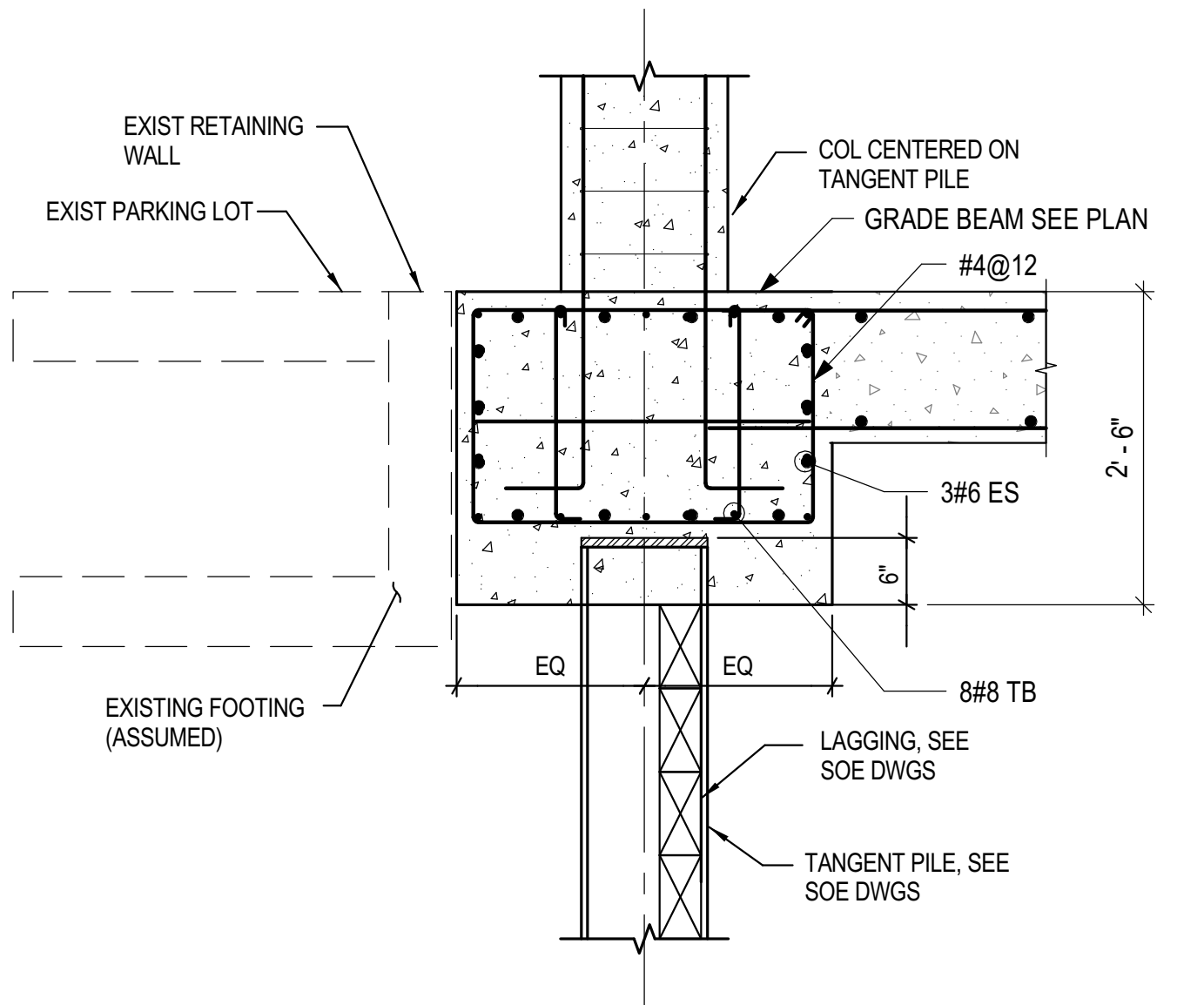
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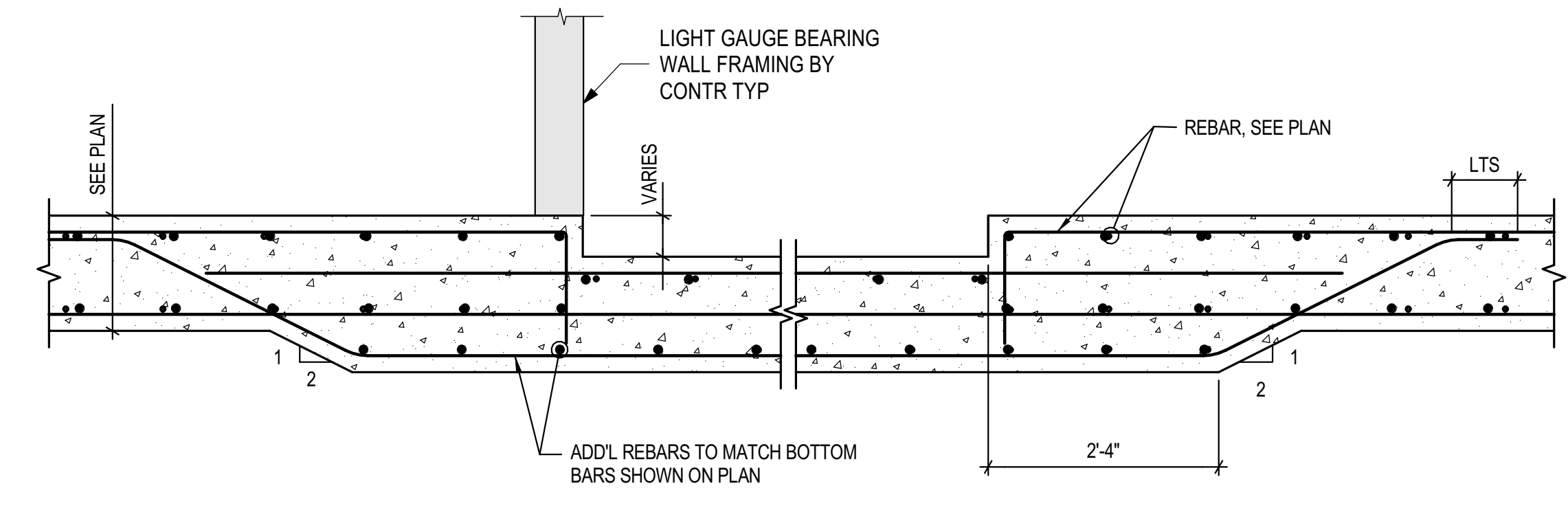
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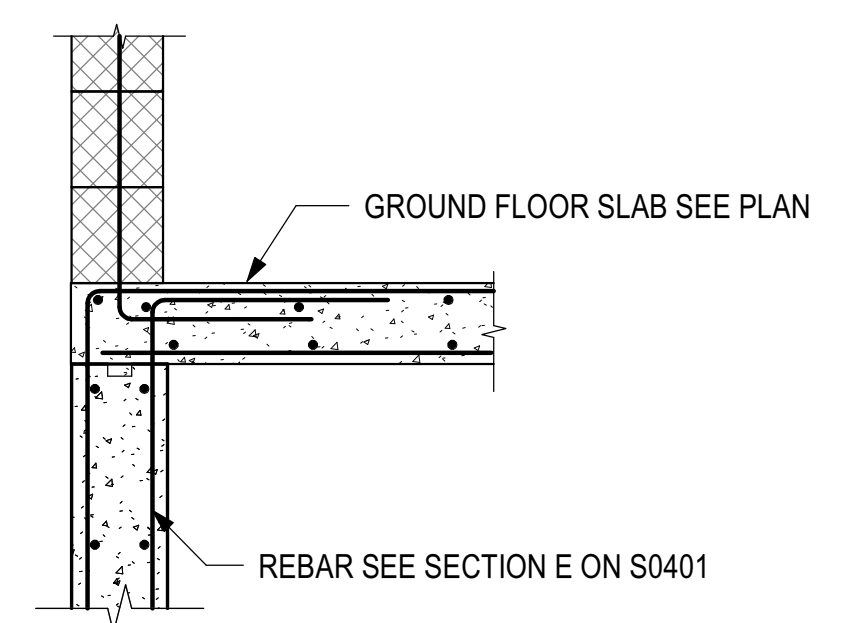
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SECTION E
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SECTION D
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SECTION G
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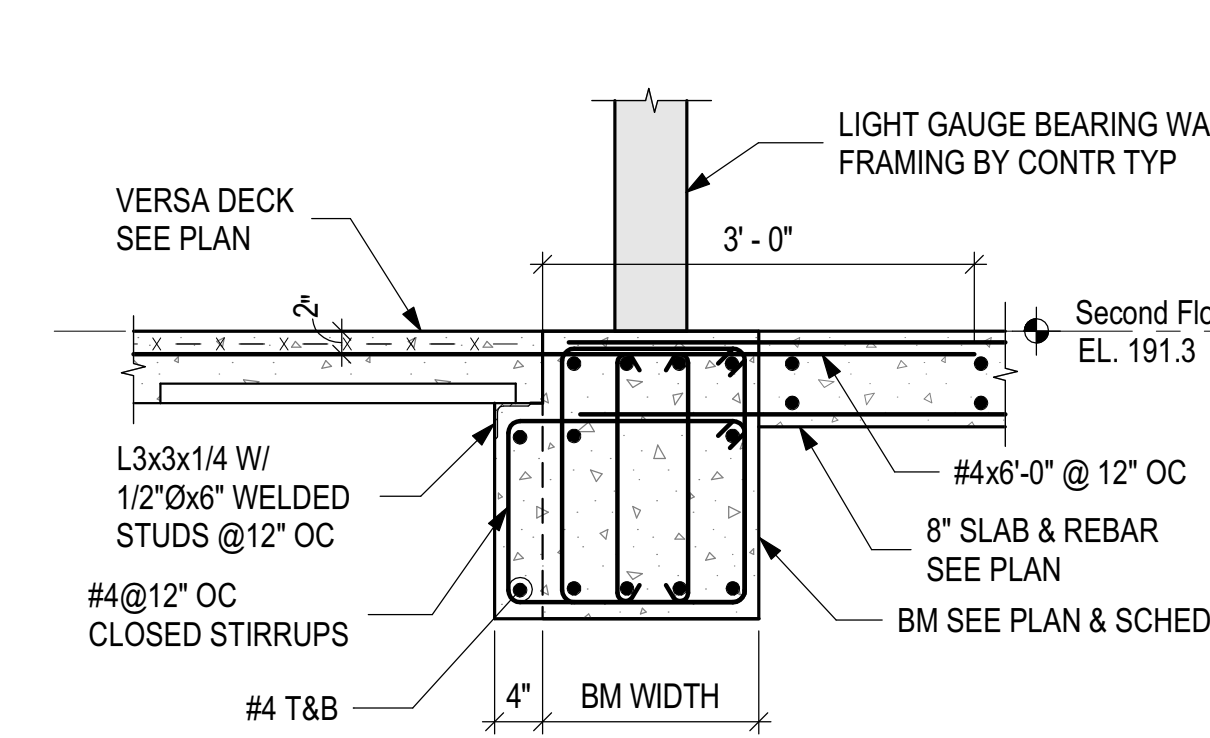
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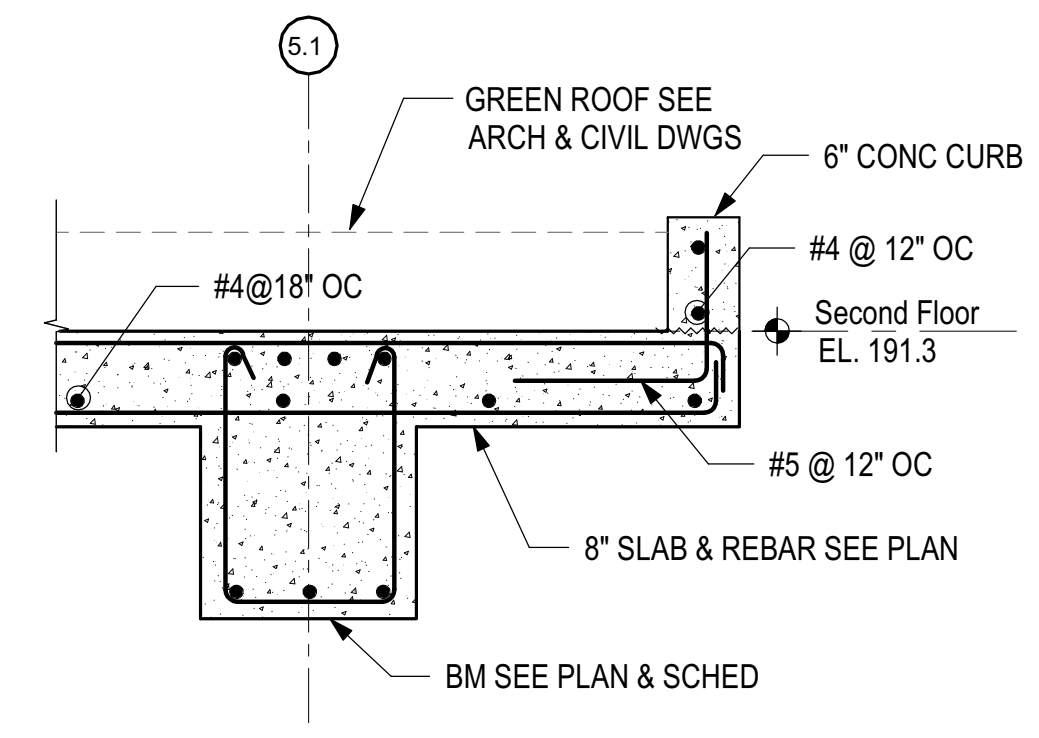
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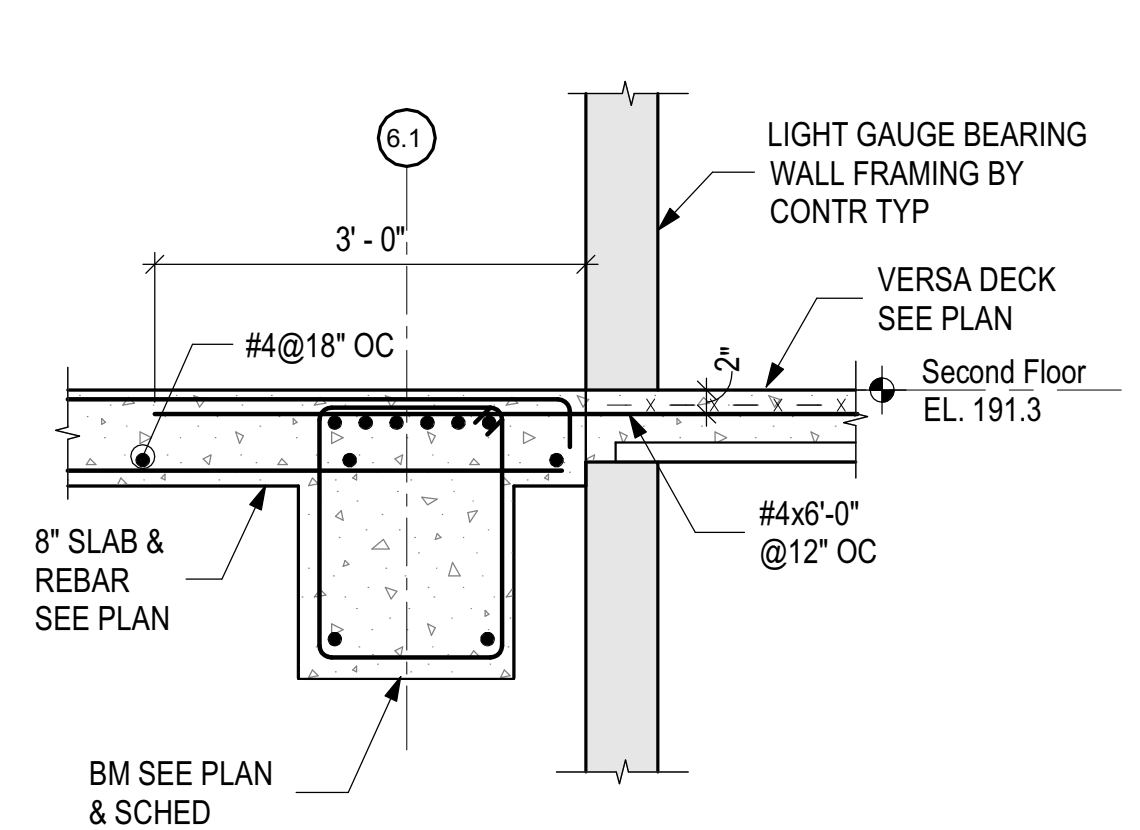
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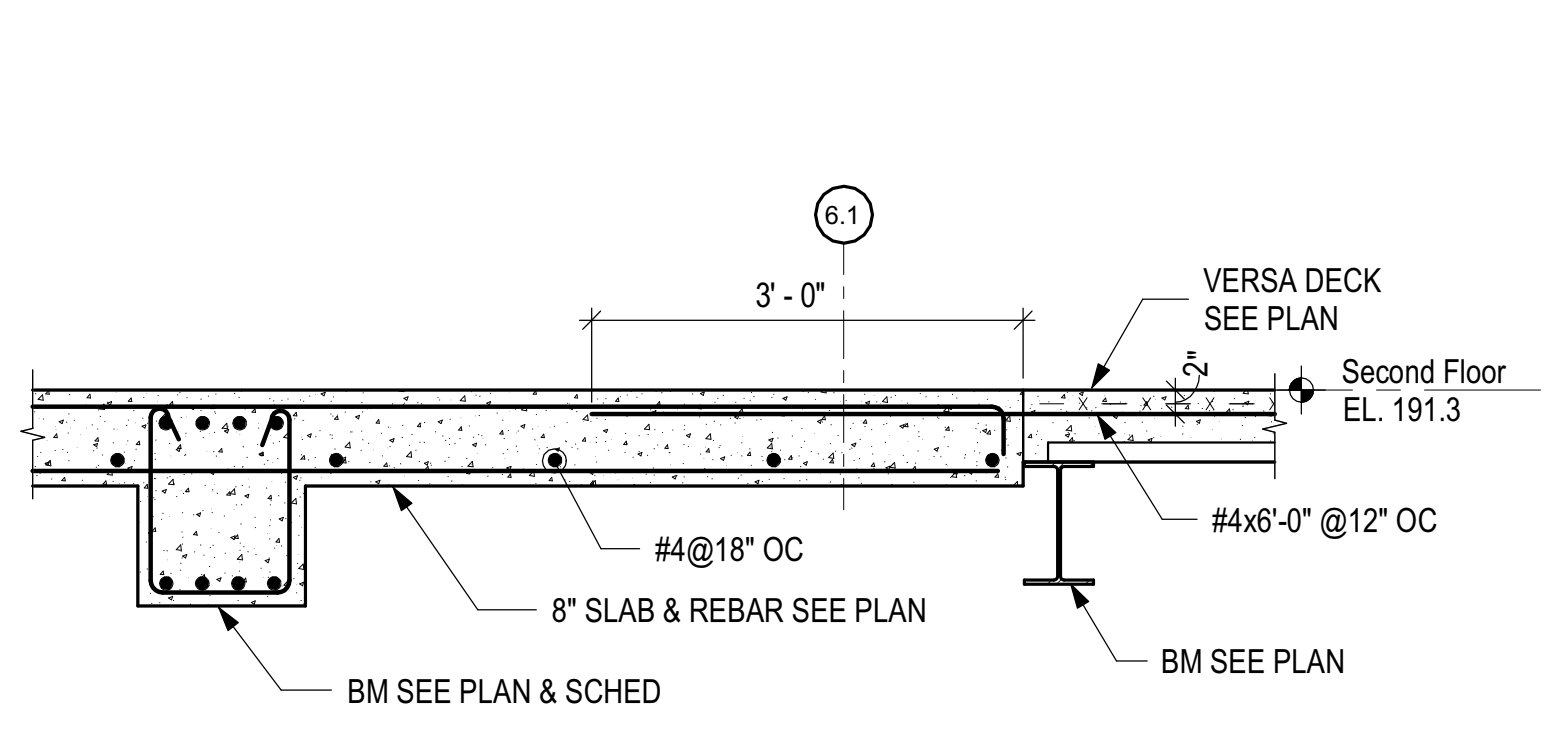
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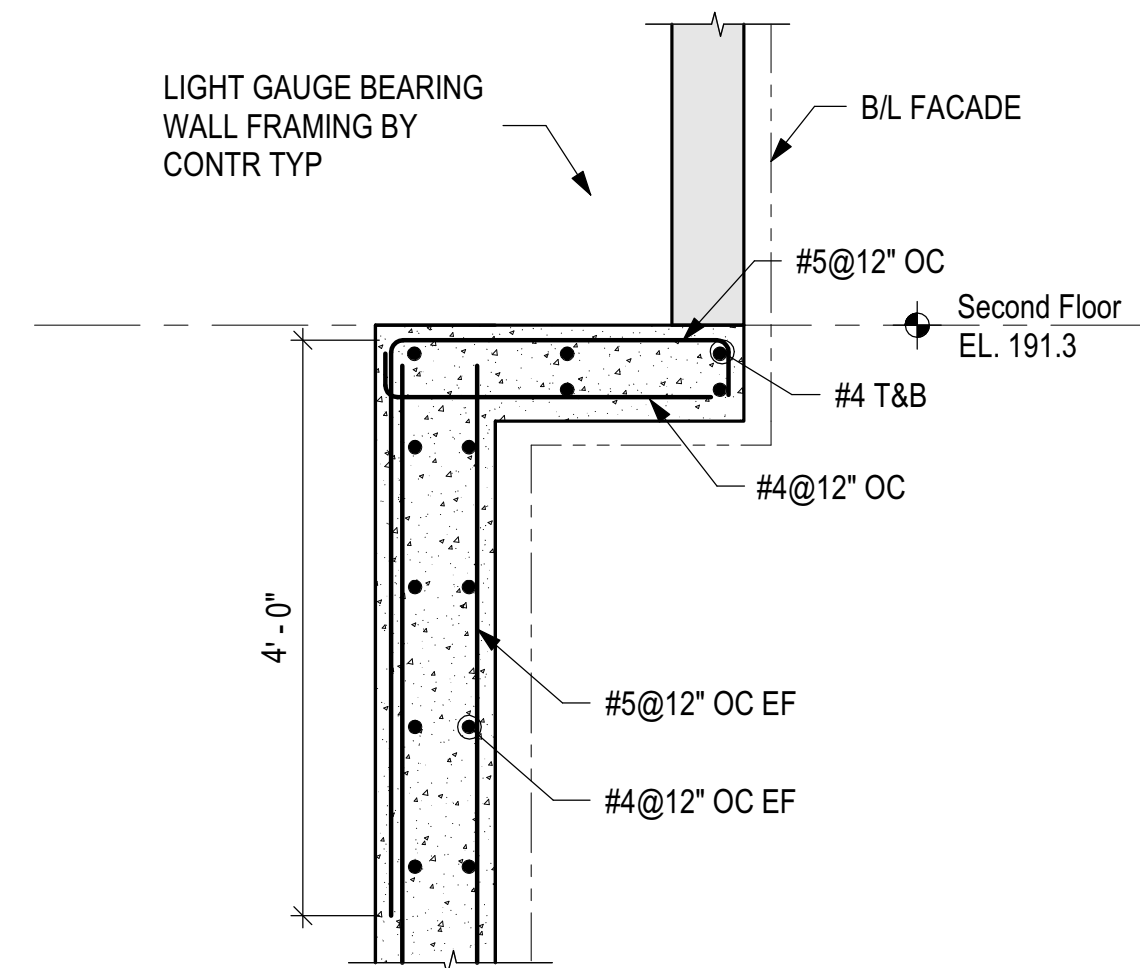
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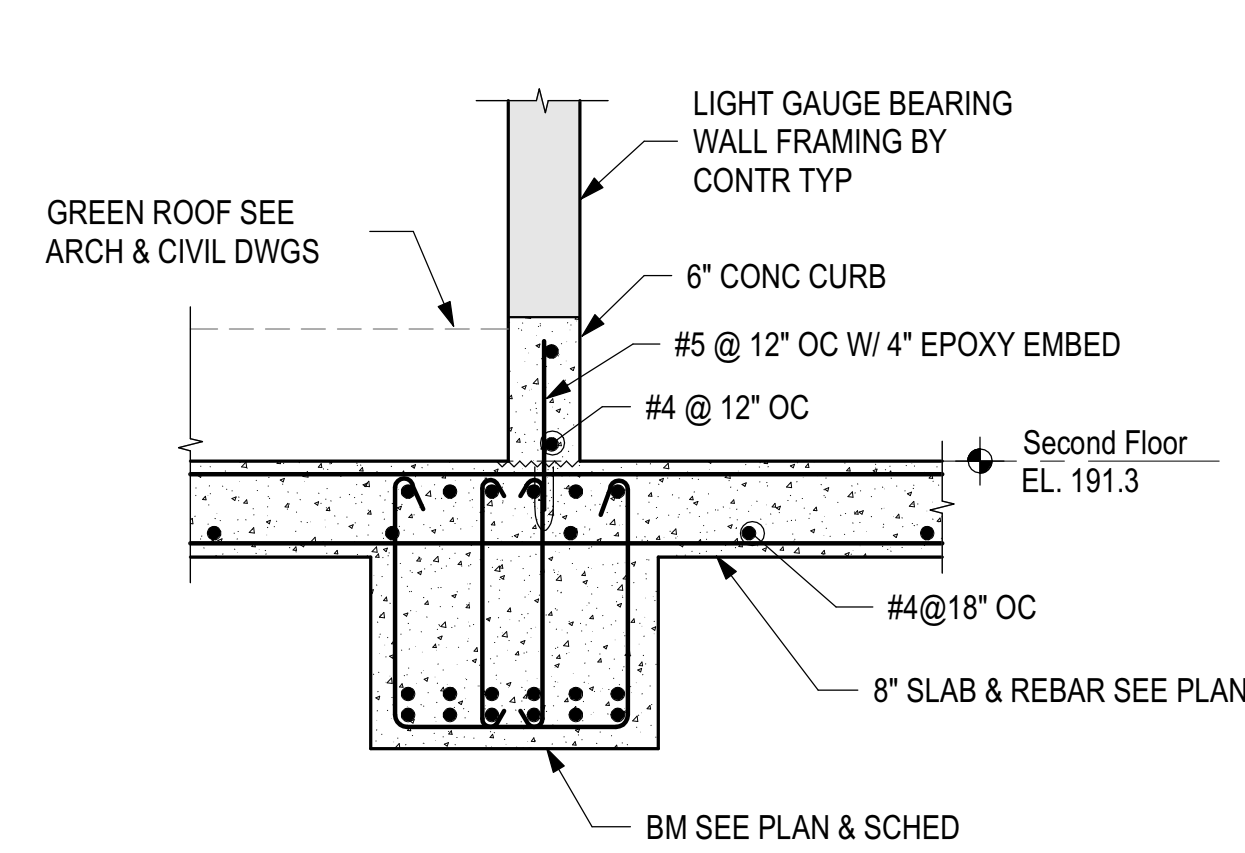
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SECTION A
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 S0403



SECTION F
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 S0403



SECTION E
 SCALE: 3/4" = 1'-0"
 S0403

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